General Comment 1 - The fragmentation of the detached rockfall masse is not included in the proposed methodology although it plays a major role for the number, size and run out of the deposited blocks, and this is critical for the rockfall hazard assessment.

Page 6 Line 11 - As also mentioned in the discussion, the fragmentation of the blocks is not taken into consideration in this analysis, however it might have a very important effect on the size and number of blocks reaching the roadway. Please comment on that.

Page 10 Line 25 - The fragmentation plays a major role on the number and size of the deposited fragments, and of course on their run out, but this is not taken into consideration here, which is one of the most important limitations of this methodology.

This is mentioned in the discussion. The purpose of this paper is to present a methodology which can be applied to combine rockfall information extracted from remote sensing with rockfall simulations to obtain a refined estimate of the hazard. The method can be extended to different types of simulations which take into account fragmentation if possible. However, many industry-standard and off the shelf rockfall simulation and modelling software does not take into account fragmentation and those that do are not doing it effectively. In addition, we do not have sufficient data such as physical tests or high temporal monitoring to calibrate models of rockfall fragmentation on this slope, therefore it has not been incorporated. It is likely that there is a larger number of smaller blocks being deposited rather than larger blocks, which is stated in the discussion. This can be elaborated on Page 10, Line 15 to 17.

General Comment 2 - The calculated percentages of Table 3 are proposed to be used in order to evaluate the most probable rockfall sources, in an inverse analysis approach. However, this approach does not take into consideration the potential effect of the rockfall source density at each buffer zone, to determine the total expected number and percentage of rock blocks reaching the track trail, ditch etc.

Page 7 Line 2 - As far as I understood, the percentages presented in Figure 9 are the percentages of the blocks reaching a certain section (track rail, ditch, past track), out of the number of the trajectory simulations for a given volume, and for all the assumed rockfall sources in a buffer zone. If this is so, in order to obtain the total percentage of the blocks reaching each location, the percentages of Fig 9 should be weighted using the percentage of potential rockfall sources at each buffer zone. As for example, it might be that in the 1st buffer zone 85% of the 10 m3 blocks reach the critical sections, but if the sources are significantly smaller that in the other buffers, the effect will be less. This fact is not taken into consideration in the proposed analysis, with a possible error on the results for the percentages of the blocks reaching the critical sections. In my opinion, the percentages presented in Figure 9, should be calculated over the total amount of simulations results for all the rockfall sources.

Page 9 Table 3 - This probabilities correspond to the all events from the sources of these zones, but according to how many potential sources each zone has, this percentage will be different. Please comment on how the probabilities that you present in this table should be interpreted.

Page 7 Line 8 - Following my previous comment, this does not mean that blocks are coming mostly from lower zones, because the number of sources might be higher in the upper zones, results in a higher absolute number of blocks on the critical sections.

Page 8 Line 13 - In order to evaluate the hazard in terms of magnitude-frequency of blocks reaching the critical sections, it is better to use the annual rockfall frequency instead of the total number of rockfalls. The number of rockfalls should also correspond to the selected buffer zone and its potential sources. This part needs further clarification.

The above comments all appear to relate to the same topic. To clarify:

- The location of rockfall sources for the period that laser scanning has taken place can be known from the change detection analysis. The problem we would like to solve is to understand how many of these rockfalls make it to the track level.
- The rockfall simulations are run, taking into account many potential source locations and the results are used to determine the likelihood that a rockfall will be deposited on the tracks or in the ditch, given its source location and block size. The graphs in Figure 9 show the results of the simulations and are not meant to show the total number of blocks ending up in a given area. They are not used
inversely to evaluate the most probable sources, as the sources are already known from the laser scanning.
- Given the results of the simulations, we are able to go back to the database of rockfalls obtained from the change detection and apply the probabilities obtained from the simulations to determine which of the known rock falls are likely to make it to the track, ditch, etc.
- We can then compare the yearly frequency of rock falls that are occurring on the slope, based on laser scanning, to the yearly frequency of rock falls deposited on the track, in the ditch, etc. (Figure 10). This summary can be included in the discussion or conclusions of the paper if necessary.

**Page 1 Line 13** - I think that you make precise here what input us obtained using the propose methodology. In my opinion, this should be made clearer throughout the text, as well.

**Page 2 Line 13** - As in my previous comment, it is not clear what are the input parameters that you mention here.

This can be clarified in the manuscript on Page 2, Line 13.

**Page 3 Line 5** - Block higher than 1 m3 that are deposited on the rails can also be of major concern as a train may crush on them. Why do you prioritize the scenario of a wedge under a train over such a crush?

In the original documentation for the CN Rockfall Hazard Rating System (RHRA), it is stated that blocks between 0.3 and 1 m are of the highest concern for operators because these blocks have the potential to become wedged under a train and derail it. This is referenced in the manuscript. Larger blocks are more likely to set off rockfall warning systems such as trip wire fences, causing trains to run at a slower speed, however the blocks between 0.3 and 1 m may often be too small to trigger any warning. Rocks larger than 1m are still a concern, but the system for this railway states that 0.3 to 1 m blocks are of specific concern. This can be detailed on Page 3, Line 9.

**Page 3 Line 29** - It would be interesting for the reader to provide some technical details, as for example the scanning distance, scanning locations, number of scans, length of slope scanned, density of the point clouds and errors.

**Page 4 Line 6** - As well, here it would be interesting to mention differences in the resolution of the different point clouds and alignment details and errors.

**Page 4 Line 12** - As for the TLS and the ALS, more details would be useful here too, on the resolution and errors of the photogrammetry obtained DTM.

Some additional details can be provided in Section 2.1. There are many details on the technical details of the laser scanning provided in other papers by the authors which have been referenced in the manuscript. The purpose of this paper is to describe a methodology for working with the information obtained from the laser scanning.

**Page 4 Line 32** - There is a very high number of small events indicated by the analysis. The calculated magnitude frequency curve suggests that about 1500 events of 0.01 m3 took place in 2 years. Is this realistic for the study area? Such a high number of events is not indicated by the inventory. Could a part of those events be attributed to alignment or resolution errors of the digital elevation models, which are compared to get the volume differences.

This is described in van Veen et al. (2017) and other papers that have been cited. A Limit of Detection threshold of 0.05 m was applied based on the alignment error between the TLS scans. In addition, other filters were applied to remove noise from the data, as described in the referenced papers. Many of the smaller rock falls have been validated using high-resolution photographs of the slope, therefore the high number of small events is realistic. This can be noted on Page 4, Line 30.
**Page 5 Line 3** - The rockfall occurrence is related to the local geological characteristics in the two opposite banks of the river. Similar discontinuity sets might have lead to rockfall in one back, but not in the other, resulting in different and not comparable magnitude-frequency curves.

This is the most complete historical dataset available for this area, therefore it is shown as the most comparable example. The two railways cross over and switch sides of the river at various points along the track in addition to changing alignment with respect to geological structures in the region. Further clarification can be added in Section 2.3 suggesting that these datasets cannot be compared directly.

**Page 6 Line 4** - It is not clear from Fig. 5b that the significant increase includes the slope 50-60 degrees. From the data it seems that slope of 60-70 degrees present instead a distinct increase. I think that the selection of the thresholds of 55 degrees needs further justification. Could the dip angle of the existing unfavourable sets could give an insight to that threshold? Additionally, in Figure 7a, you also differentiate in the legend between slope angles of 55-75 degrees and >75 degrees. Why? This distinction in not included in the text.

Figure 7a can be updated to remove this distinction. By adding lines to figure 5b it is more clear that the change in slope angle occurs between the 50-60 degree bin and the 60-70 degree bin. Given the relatively short sampling period of data for this slope, we must be slightly conservative. Therefore 55 degrees has been used. This can be further justified in the manuscript. See image below.

**Page 6 Line 14** - Why do you use the minimum deposited volume, instead of the maximum one which is a worse scenario?

The minimum deposited volume is shown, as any block size larger than this will also be deposited in these locations. Therefore, by showing the minimum volume, we can understand the full range of block sizes that could be deposited in each location. This can be specified on Page 6, Line 15.

**Page 7 Line 1** - In Figure 9, why rockfall sources at a distance of 350-500 m from the trail, result in more blocks deposited in the ditch, than the sources of 300-350 distance? In all the other cases, but this, more blocks reach the ditch if the source is closer to the track trail.
The graphs show a percentage, and not an absolute number of blocks being deposited at the base of the slope, as we are concerned with a probability of a rock reaching the track or the dith. The graphs show that a larger percentage of blocks sourced from 350-400 m are reaching compared to blocks sourced from 300-350 because there are relatively few source zones between 350-400 m, and because of where these source zones are located, a larger proportion of the blocks travel further, compared to the 300-350 distance where there is a larger spatial variation in source zones, so the slope geometry has more of an overall effect. The total number of blocks sourced from 300-350 reaching the base of the slope is larger than 350-400, however the relative proportion is smaller. This can be commented on in the discussion if necessary.

Page 8 Line 12 - How do you define the size the thresholds of the bins?
Page 8, Line 11 and 12 can be reworded/clarified. Instead of specifying that the modelled volume is the central point of each bin, we could state that each of the rock falls was assigned a probability based on being rounded to the closest modelled volume. We also propose to add a diagram to further show the different bins.

Page 10 Line 18 - This is not certain. Rockyfor3D can produce this output.
We state that it is not possible to track the location and energy of a block at any given point in time to determine the specific passage of each block down the slope. In RockyFor3D it is only possible to determine the energy statistics at a given cell in the raster, not for a particular, individual rock fall event. This can be specified on Page 10, Line 18.
Section 2.1: Please give some more information on the point cloud data like scan settings, achieved point density and alignment error. I expect that TLS and ALS data show quite different point densities. You talk about the creation of a surface/slope model. From Fig. 2 it seems it is a raster model. Please add more information about how this model was created (interpolation method) and which cell size was used. The same applies to the photogrammetric model, please add details.

The point densities of the models can be provided in Section 2.1. The TLS and combined TLS/ALS slope models refer to the point cloud data generated from the LiDAR scans. This can also be specified. The slope model was converted into a raster DEM using an inverse distance weighted interpolation and 1m grid size, as specified in the paper. Details on the scan settings, scan processing, and photogrammetric model generation are provided in many of the papers that have been cited. Some further details could be added to Section 2.1.

Section 2.2: You cite Jolivet et al. (2015) regarding the methods used for classification. Please add some more information on what these methods are, especially as this report seems to be difficult to access.

A reference can be added on Page 4, Line 18, to another paper by the author (van Veen et al., 2016) which describes the methodology in more detail, also referencing Jolivet et al. (2015) who originally developed this method.

Figure 4: why are there gaps in the maps?

The gaps are occlusions in the dataset. This can be noted in the caption to Figure 4.

Section 2.3: You cite van Veen et al. (2017) regarding the applied (semi-automatic) change detection methods. Please add some more information: which method was used to calculate distances/volumes (2D/3D, raster or point based)? How were individual locations detected/delimited? Again, it would be interesting on which cell size these operations were performed (see above).

The operation was performed using 3D point based methods as described in detail in van Veen et al. (2017) and other papers by the authors which have been cited. They have not been included in this paper as the focus of this paper is the presentation of the methodology for taking the extracted rockfall data and using the simulations to provide a refined estimate of the rockfall hazard at track level. A brief summary can be added to Page 4, Line 27.

General comment - Regarding the high number of small events in the TLS data – might this be related to the alignment accuracy of the datasets (see above)? Did you apply a level of detection (LOD) threshold to distinguish between real change and measurement error?

This is also described in van Veen et al. (2017) and other papers that have been cited. A LOD of 0.05 m was applied based on the alignment error. In addition, other filters were applied to remove noise from the data, as described in the referenced papers. Some smaller rock falls have been validated using high-resolution photographs of the slope, therefore the small events are real. This can be noted on Page 4, Line 30.

General comment - Regarding rockfall locations and volume: are there any relationships between volume and GSI?

There is not a clear pattern between the rockfall locations/volumes and GSI. In general, the lowest GSI range produces the most rockfalls and largest GSI produces the fewest rock falls, but in between the relationships is not as clear. This analysis is outside the scope of this paper therefore no changes are proposed.

Section 3.3: Figure 8: why do you show the minimum deposited volume (and not the worst case)? To make the differences in travel length with regard to block size more obvious? Please comment.
The minimum deposited volume is shown, as any block size larger than this will also be deposited in these locations. Therefore, by showing the minimum volume, we can understand the full range of block sizes that could be deposited in each location. This can be specified on Page 6, Line 15.

**Figure 9:** there seems to be a more or less strict order regarding the distance of the source zone from the track and the percentage of rockfalls deposited. Did you investigate if this could be related to the simulation model assumptions? I would have expected that GSI/roughness and changes in slope could result in a more heterogeneous result. Is it really just that there are only fewer complex geometrical features and fewer zones of talus present in the nearer sections?

It is expected that there would be a strong relationship between the distance of the source zone from the track and percentage of rockfalls deposited. It is possible that the local slope roughness does impact the results, and that there may be certain areas of the slope where the roughness or slope angles play a larger role than others, but the distance from the track is the controlling factor in the overall percentage of rockfalls making it to the track. A detailed sensitivity analysis of the modelling parameters is beyond the scope of this study. This can be made clear in the objectives (Section 1.1).

**Section 4.1:** How did you determine the bin size? In the example for the 0.1m3 volume class you use a bin range from 0.05m3 to 0.5m3. How does that match with your statement that the modelled volume is the central point of each bin? Is the bin size constant or differs the size for each modelled volume? Please comment.

This can be reworded/clarified on Page 8, Line 11 and 12. Instead of specifying that the modelled volume is the central point of each bin, we could state that each of the rock falls was assigned a probability based on being rounded to the closest modelled volume. We also propose to add a diagram to further show the different bins.

**Section 5/6:** Regarding rockfall fragmentation: from change detection you also know at which locations material has been accumulated. Did you balance the erosion / deposition volumes, and if so, how much do they differ? Did you compare the total area covered by either erosion or deposition? That might give a hint on fragmentation.

At this site, it is difficult to match areas of material accumulation to individual rockfall events for several reasons. The talus piles at track level are cleared out regularly by railway operating crews and result in negative change along the base of the tracks. In addition, when then rock falls fragment, sometimes the fragments spread out and the accumulation is below the limit of detection. This is why we are investigating the use of simulations at this site to determine what percentage of rockfalls will make it to the track level, as it is difficult to do so otherwise.

**The following comments are understood and will be addressed/corrected:**

In general, I think the introduction could be elaborated – references to previous studies/approaches and some comments in which aspects this paper goes beyond.

A minor comment: you mostly write “frequency-magnitude” relationship throughout the text, but also use “magnitude-cumulative frequency (MCF) curve”. I think the usage of “magnitude-frequency” relationship is more common, so it might be worth to homogenize this throughout the manuscript.

Please provide a small note (and not only in the discussion) that such a short time period of only 18 months may be problematic in order to establish a magnitude-frequency relationship. This also applies to the differences seen compared to the historical inventory (large events may be missing).

Technical corrections:

p. 8, l. 31: I think the “and” should be removed
p.12, l. 30: add missing line break
p. 13, l. 17: fix typo in "frequency"
Figure 1b: please add a scale bar
Combining temporal 3D remote sensing data with spatial rockfall simulations for improved understanding of hazardous slopes within rail corridors

Megan van Veen¹,², D. Jean Hutchinson¹, David A. Bonneau¹, Zac Sala¹, Matthew Ondercin¹,², and Matt Lato¹,²

¹Department of Geological Sciences and Geological Engineering, Queen’s University, Kingston, K7L 3N6, Canada
²BGC Engineering Inc., Toronto, M5H 2W9, Canada

Correspondence to: Megan van Veen (mvanveen@bgengineering.ca)

Abstract

Remote sensing techniques can be used to gain a more detailed understanding of hazardous rock slopes along railway corridors that would otherwise be inaccessible. Multiple datasets can be used to identify changes over time and create an inventory of events including frequency-magnitude relationships for rockfalls sourced on the slope. This study presents a method for using the remotely sensed data to develop inputs to rockfall simulations, including rockfall source locations and slope material parameters, which can be used to determine the likelihood of a rockfall impacting the railway tracks given its source zone location and volume. The results of the simulations can be related to the rockfall inventory to develop modified magnitude-frequency curves presenting a more realistic estimate of the hazard. These methods were developed using the RockyFor3D software and LiDAR and photogrammetry data collected over several years at the White Canyon, British Columbia, Canada, where the CN Rail main line runs along the base of the slope. Rockfalls sourced closer to the tracks were more likely to be deposited on the track or in the ditch, and of these, rockfalls between 0.1 and 10 m³ were the most likely to be deposited. Smaller blocks did not travel far enough to reach the bottom of the slope and larger blocks were deposited past the tracks. Applying the results of the simulations to a database of over 2000 rockfall events, a modified magnitude-frequency curve can be created, allowing the frequency of rockfalls deposited on the railway tracks or in ditches to be determined. Suggestions are made for future development of the methods including refinement of input parameters and extension to other modelling packages.

1 Introduction

Railways in Western Canada are subject to frequent rockfall hazards which can be evaluated using remotely sensed data, derived from LiDAR or structure from motion (SfM) photogrammetry. Often, these technologies provide advantages over traditional methods of data collection on steep and inaccessible slopes, such as the ability to identify rockfall release points, which can be a large source of uncertainty in developing rockfall magnitude-frequency relationships (Corominas et al., 2018).
Change detection performed between sequential LiDAR scans can be used to identify and characterize rockfall source zones and develop magnitude-frequency relationships for a slope (Rosser et al., 2005; Santana et al., 2012; D'Amato et al., 2013; Guerin et al., 2014; van Veen et al., 2017). These inventories can contain a greater level of detail for rockfalls in the smaller volume ranges relative to traditional field inspections performed from the base of the slope (Dussauge-Peisser et al., 2002); however, it is often difficult to determine the trajectory or ending point of each event, which is affected by the slope materials and geometry, and the characteristics of the failed block, including its lithology (Deere and Miller, 1966). While many have demonstrated the ability to develop rockfall magnitude-frequency relationships using remote sensing data, these inventories likely overestimate the likelihood of a block interacting with critical infrastructure at the base of a slope. Evaluation of rockfall trajectories can provide insight into the potential behaviour of rockfalls, which can be used, along with the magnitude-frequency and rockfall source zone information, in hazard assessment and mitigation design for a given section of railway.

Along with the locations and characteristics of rockfall source zones, remote sensing data can also provide us with detailed surface models for use in rockfall modelling environments. The use of 2.5-D and 3D environments for computer based rockfall modelling provides advantages over traditional 2D methods as the entire slope can be examined without having to draw cross-sections through selected areas and model along only those paths (Lan et al., 2010; Ondercin et al., 2014).

1.1 Study Objectives

This study investigates how detailed remote sensing data of multiple types can be incorporated into rockfall simulations, to create representative surface models and to develop input parameters for rockfall source zones and slope characteristics. The input parameters extracted from the remote sensing data include rockfall sources and source zone characteristics (ex. lithology, slope angle) in addition to detailed slope classifications used to assign material properties. We compare several different cases based on rockfall volumes and source zone locations using RockyFor3D, a widely available industry standard 2.5D point mass model (Dorren, 2015). We present a method which uses the results of the rockfall simulations to determine the likelihood of a rockfall impacting the railway tracks given its volume and source zone location. These results can be used to modify the magnitude-frequency curves for rockfall source zones (developed using LiDAR data) to develop a more refined estimate of the hazard likelihood.

1.2 Study Site

The site of interest is known as the White Canyon, and is located approximately 250 km northeast of Vancouver, near the town of Lytton, BC (Fig. 1). The CN Rail line runs along the base of the slope, which is separated into two sections by a short tunnel, White Canyon West and East. The work presented in this paper focuses on the White Canyon West section. Due to differential weathering of several lithological units, the canyon contains many deeply incised channels which are actively transporting debris downslope predominantly by dry granular flows and debris flows (Bonneau and Hutchinson, 2017). Surrounding these channels are areas of more competent rock outcrop, which act as source zones for rockfall. Many of these areas contain large vertical spires, contributing to the complex geometry of the slope (Fig. 1) and affecting the passage of rockfalls from higher
source elevations to the railway tracks or into the river at the bottom of the slope. For this reason, the ability to model rockfalls on this slope in more than two dimensions is advantageous.

The slope is composed of a highly fractured and foliated quartzofeldspathic gneiss with a series of tonalite dykes and dioritic intrusions. A section of red-stained and highly weathered granodiorite from the Mt. Lytton Batholith is also present in a small section of the slope (Brown, 1981). The degree of fracturing is variable throughout the canyon, leading to a variety of rockfall block shapes and sizes within each unit. The amount of weathering is also variable across the slope, and affects the behaviour of rockfalls.

The high frequency of rockfalls on this slope presents a challenge to the rail operators. Typically, track-level assessments by CN Rail provide records that can be added to a database of rockfall events for a given slope and rail corridor. These could include events that have directly impacted the track or blocks that are noted in ditches adjacent to the rail. Using CN’s Rockfall Hazard Rating System (RHRA, (Abbott et al., 1998b)), events are categorized into three volume classes based on the largest block dimension at track level (less than 0.3 m, 0.3 to 1.0 m, and greater than 1.0 m). Of these sizes, the blocks between 0.3 and 1.0 m in size present the greatest concern as they are the size most likely to become wedged under a train, lifting it off the tracks and causing a derailment (Abbott et al., 1998a). Smaller blocks may not present as significant of a hazard and larger blocks are more likely to set off rockfall warning systems such as trip wire fences. For slopes where activity is frequent, such as the White Canyon, these inspection-based databases are often incomplete as it is difficult to identify individual small events that may occur between inspections as a single rockfall.

As a part of the Canadian Railway Ground Hazards Research Program, remote sensing data, including terrestrial laser scanning (TLS), airborne laser scanning (ALS), and SfM photogrammetry data, have been collected at the site since 2012. Several studies have been completed using this data with details outlined in Hutchinson et al. (2015), Kromer et al. (2015), van Veen et al. (2017), and Rowe et al. (2017). By performing change detection between sequential LiDAR scans, we have constructed a detailed database of rockfall events in the canyon for the time period between November 2014 and May 2016 (18 months).

2 Data Collection, Processing and Classification

Data were collected over a period of 18 months for the White Canyon West slopes. The data were processed to create models of the full slope which were classified to develop inputs to the rockfall simulations. Change detection methods were used to extract rockfall event information from the data and develop magnitude-frequency curves for the analysis period.
2.1 Remote Sensing Data Collection and Creation of Slope Models

The TLS data for the White Canyon West slopes used in this study were collected on eight dates between November 2014 and May 2016. The duration between collection dates ranged from one to four months and was dependent on site access and weather conditions. Data were collected using an Optech ILRIS-3D scanner from five different sites on the opposite bank of the Thompson River (Fig. 1). The survey was designed such that there was significant overlap between sites, in order to minimize the amount of data occluded from the overall slope model. **TLS data were collected at a distance of 300 to 700 m from the slope with an average point spacing of 10 cm.** ALS data (point spacing ranging from 30 to 60 cm) were collected in October of 2015, one week prior to a TLS data collection campaign, and was provided by CN Rail.

Sequential TLS datasets can be aligned to one another by picking common points between the models and then performing an Iterative Closest Point (ICP) alignment (Besl and McKay, 1992). A single slope model was created for each of the eight TLS scan dates by combining the data from each of the five individual scan sites. Each completed TLS model was aligned to the baseline reference model collected in November 2014. Due to the complex geometry of the slope, both the TLS and ALS data contain occlusions, however combination of both models provides near-complete coverage of the slope. In order to create a single model of the slope to be used as a modelling surface, the ALS data were aligned to the TLS data from October 2015 (datasets collected approximately one week apart). The resulting slope model is shown in Fig. 2. The result of this process is a model of the slope with reduced occlusions compared to the TLS model on its own.

A set of overlapping photos of the slope was collected on September 11th 2013 using a Canon 6D camera with a 50 mm lens. The 94 photos were used to create a photogrammetric model of the slope using Agisoft PhotoScan Pro (Agisoft, 2015) with a point spacing of 10 to 20 cm. This model was aligned to the LiDAR reference model using the same methods described above.

2.2 Classification of Slope Models

The slope was classified into each of the four major lithological units present using the photogrammetric model (Fig. 3): Quartzofeldspathic Gneiss (primary rock type), Mt. Lytton Batholith (granodiorite), tonalite dykes, and dioritic intrusions. This classification was performed through visual inspection and tracing of each outcrop area on the 3D model using the methods and preliminary work developed by Jolivet et al. (2015) and described in van Veen et al. (2016). The same process was used to separate the slope into areas of rock outcrop (potential rockfall source zones) and areas of soil and talus, and to classify the slope by visual assessment of the Geological Strength Index (GSI) (Marinos and Hoek, 2000). GSI values were assigned by visually comparing high resolution photographs of the rock mass with the GSI characterization table for jointed rock. Sections of the rock mass were assigned a GSI value based on the rock mass structure and conditions of discontinuities observed on the outcrop surface. The classified slope model for each of the categories described above is shown in Fig. 4.
2.3 Extraction of Rockfall Magnitude-Frequency and Source Zone Information

Rockfall locations and volumes were extracted from the TLS change detection using 3D point-based methods and spatial clustering algorithms as described in van Veen et al. (2017). A limit of detection threshold of 0.05 m was applied based on the alignment error between sequential TLS datasets and additional noise filters were applied during data processing. Results were validated using high resolution photos collected simultaneously with the TLS data. Using the rockfall locations, each event could also be related to its source zone lithology and GSI using the classified models, as well as the source zone slope angle for each event. We created an inventory of events, using the eight TLS datasets that were collected between November 2014 and May 2016 (18 months). This inventory was used to produce a magnitude-cumulative frequency (MCF) curve and spatial distribution plot for the rockfalls on the slope during this time period (Fig. 5). While a short time period of only 18 months may not be sufficient to establish magnitude-frequency relationships for larger rockfall events, it provides enough detail to understand the smaller magnitude hazards which affect the day-to-day operation of the railway.

Hungr et al. (1999) compiled a rockfall inventory over the previous four decades along the Canadian Pacific rail line and BC Highway 1 corridor from Vancouver to Kamloops. The CP line generally follows the same route as CN throughout this corridor but is located on the opposite bank of the Thompson and Fraser rivers. These historical records contain data for events where volume information was available and span magnitudes from 0.01 to 3000 m³ with a noted under sampling of events below 1 m³. While this inventory can’t be directly compared to the inventory generated using TLS, due to the different sampling periods and variation in route of the highways and railways, the TLS analysis methods provide a higher level of detail for rockfalls in the range of 0.01 to 1 m³ compared to the historical inventory in this area. In addition to a higher level of detail for smaller volume ranges, the rockfall information gathered from LiDAR provides information on the location of rockfall source zones, which can be used to characterize the failure processes operating on the slope. This information is not available from historical records collected at the base of a slope.

3 Rockfall Simulations – RockyFor3D

The RockyFor3D model (Dorren, 2015) is a 2.5D point mass model used to calculate rockfall trajectories as 3D vector data. RockyFor3D is an industry standard software model that utilizes GIS data formats for model inputs, outputs, and visualization. This model was used to simulate rockfalls ranging from 0.01 to 100 m³ across the entire slope of the White Canyon West. The classified slope models were used to determine input parameters for the RockyFor3D modelling. The results of these models allowed us to determine the percentage of the simulated rockfalls that landed in the ditch at the base of the slope, on the railway tracks, or were deposited past the tracks, for a given source zone location and volume.
3.1 Input Parameters

The RockyFor3D model requires 10 different input parameters which are summarized in Table 1. The Digital Elevation Model (DEM) was created from the merged TLS/ALS dataset from the canyon using an inverse distance weighted interpolation in ArcGIS and the rg70, rg20, rg10 and soiltype parameters were estimated based on the classified slope models. The rg70, rg20 and rg10 parameters represent the “roughness” of the slope within each DEM cell by specifying the height of obstacles that a rockfall will encounter within each 1 m cell of the DEM in 70%, 20% and 10% of cases (Dorren, 2015). The surface roughness parameters for areas of rock outcrop were estimated based on the GSI classification for each slope area.

Table 1. Summary of input parameters for RockyFor3D models

<table>
<thead>
<tr>
<th>Input/Parameter</th>
<th>Description</th>
<th>Input Data or Value(s)</th>
</tr>
</thead>
<tbody>
<tr>
<td>dem</td>
<td>Digital elevation model – 2.5D surface for modelling</td>
<td>1 m grid DEM from LiDAR data (merged TLS and ALS data from October 2016)</td>
</tr>
<tr>
<td>rockdensity</td>
<td>Density of the rock</td>
<td>2700 kg/m$^3$ (assumed to be constant for the entire slope)</td>
</tr>
<tr>
<td>$d_1$, $d_2$, $d_3$</td>
<td>Dimensions of falling block in metres</td>
<td>Assumed that $d_1=d_2=d_3$ (cube shaped rockfall). Different models run with volumes of 0.01, 0.1, 1, 10 m$^3$.</td>
</tr>
<tr>
<td>blshape</td>
<td>Shape of the rock block – choice between rectangular, ellipsoidal, spherical or disk shaped</td>
<td>“Rectangular” – will be cubic since $d_1=d_2=d_3$. Selected based on highly angular nature of rocks in the White Canyon.</td>
</tr>
<tr>
<td>$rg_70$, $rg_20$, $rg_{10}$</td>
<td>Defines surface roughness within each 1m DEM cell, used to determine tangential restitution</td>
<td>Based on GSI for areas of rock outcrop. Values for each GSI estimated from high resolution photos (Fig. 6). Assumed a constant roughness ($rg_{70}=rg_{20}=rg_{10}$) of 0.15 m for talus channels.</td>
</tr>
<tr>
<td>soiltype</td>
<td>Values used to determine normal restitution. Selected from a list of seven values each corresponding to a different coefficient of restitution (COR).</td>
<td>Assigned based on ground cover and lithology classification. Talus assigned lowest value, gneiss and granodiorite assigned intermediate value, and more competent intrusions and dykes assigned highest value. Range between soiltype 4-6 (COR range from 0.38 to 0.53).</td>
</tr>
</tbody>
</table>
3.2 Modelling Process

The four different rockfall volumes, ranging from 0.01 to 10 m³ were sourced from grid cells where the slope angle exceeded 55° (Fig. 5). The threshold of 55° was selected based on an analysis of the slope angle at known rockfall source points (Fig. 5). When comparing the number of rockfalls per unit area of different slope angle ranges, there was a distinct increase in the relative rate of rockfalls for slope angles between 50-60° and greater. While it may be argued that this increase occurs at a larger angle, a conservative approach was taken, given the relatively short time period used to produce this dataset. The rockfall source zones were selected using this logic as opposed to sourcing only from known rockfall event locations (within the 18-month period) as to better capture all of the potential source zones on the slope. 500 blocks were sourced from each of these cells. One model was run for each volume category and source distance from the railway track (total of 40 models, Fig. 7). Smaller zones (25 m distance increments) were used for the first 100 m, and 50 m buffered contours were used for distances greater than 100 m, such that the results would contain more detail for rockfalls occurring closer to the tracks.

3.3 Results

The results from the RockyFor3D simulations were combined and used to produce raster maps of rockfall deposition locations for each of the source zone distance ranges, an example of which is shown in Fig. 8. These raster maps display the minimum volume that was deposited in each cell based on the simulations. Any block larger than the size shown can also be deposited in these locations, therefore showing the minimum volume allows for an understanding of the full range of block sizes that could be deposited in each location. The rockfall deposition points were classified and used to determine the percentage of rockfalls being deposited in a given location based on the assumptions made for the model inputs. The deposition percentages were split into rockfalls deposited in the ditch, on the railway tracks, below track level, and the percentage remaining on the slope for each volume and source distance (Fig. 9). The general trends observed in the deposition results are summarized in Table 2.

<table>
<thead>
<tr>
<th>Deposition Locations</th>
<th>Trends Observed</th>
</tr>
</thead>
<tbody>
<tr>
<td>Rockfalls deposited in ditch, on track or past tracks</td>
<td>• Generally, shows an increase in deposition percentage for increasing volumes.</td>
</tr>
<tr>
<td>(rockfalls making it past the slope, Fig. 9a)</td>
<td>• Shows a decrease in deposition percentage for source zones further from the tracks.</td>
</tr>
<tr>
<td>Rockfalls stopping on track or in ditch (Fig. 9b)</td>
<td>• Does not exceed 20% deposition except for source zones less than 25 m from tracks.</td>
</tr>
<tr>
<td></td>
<td>• Generally, the highest deposition percentage is seen for 1 m³ or 10 m³ volumes, depending on source locations (some larger blocks travel further, past tracks).</td>
</tr>
</tbody>
</table>

Table 2. Summary of trends observed from RockyFor3D modelling deposition results.
For rockfall source zones less than 25 m from the tracks, the percentage of events deposited in the ditch or on the tracks was 20 to 30% higher than rockfalls sourced at larger distances. In the lower sections of the slope, there are fewer complex geometrical features and fewer zones of talus present, which provides a more direct passage for rockfalls to be deposited on or near the tracks. Smaller volumes were more likely to be deposited in the ditches than larger volumes, and the 1 and 10 m³ blocks were most likely to be deposited on the tracks. It is possible that the local slope roughness does impact the results, and that there may be certain areas of the slope where the roughness or slope angles play a larger role than others, however the distance from the track appears to be the controlling factor in the overall percentage of rockfall making it to track level. For rockfalls sourced from the uppermost slopes, the results show that a larger percentage of blocks sourced from 350-400 m are reaching the base of the slope compared to blocks sourced from 300-350 m. This result occurs because there are relatively few potential source zones between 350 and 400 m. Because of where these source zones are located, a larger proportion of the blocks travel further, compared to the 300-350 distance where there is a larger spatial variation in source zones, and the slope geometry has more of an overall effect. The total number of blocks sourced from 300-350 m reaching the base of the slope is larger than 350-400 m, however the relative proportion is smaller.

4 Application of Results to the Rockfall Hazard Inventory

4.1 Refinement of Hazard Likelihood

The results of the RockyFor3D models (deposition percentages) were applied to the database of rockfalls identified from TLS data to reproduce the rockfall MCF curves for several different scenarios (Fig. 10):

- Rockfalls landing in the ditch or on the tracks (what would typically go into an event database)
- Rockfalls landing on the tracks (blocks that present a derailment hazard)
• Rockfalls landing in the ditch (and therefore require maintenance activities in order to be cleared to provide future rockfall collection capacity)

This adjustment was made based on the likelihood of a rockfall being deposited in a given location given its source zone location. For rockfall source zones in each given distance and volume range, the percentage of rockfalls deposited in a certain location was used to subsample the rockfall database. The inventory of rockfall volumes was split into bins, with the modelled rockfall volumes as the central point of each bin. For example, if the percentage of rockfalls making it to track based on the model was 50% for the 0.1 m³ volume class and 50-75 distance range, and there were 200 rockfalls between 0.05 and 0.5 m³ in this range, then 100 of these events would be randomly subsampled for the adjusted database.

The rockfall inventory based on TLS data was split into groups based on source zone distance from the tracks and rockfall volume. Rockfalls were assigned a volume group by rounding the volume of each event to the closest modelled volume (for example, a rockfall with volume 0.7 m³ would be assigned to the 1 m³ volume group). Each group was randomly subsampled based on the probability of a rockfall with that source distance and volume making it to the track or ditch, and the subsampled inventory was used to develop refined MCF curves for each scenario.

In comparing the reproduced MCF curves for rockfalls deposited on tracks or in the ditch to the rockfall sources identified through TLS change detection, the yearly cumulative rockfall frequencies are up to an order of magnitude lower. The difference between the yearly frequencies becomes smaller as the rockfall magnitude increases, as these blocks have a higher probability of travelling to the base of the slope. In comparing the frequency of rockfalls deposited in the ditch or on the tracks to the frequency reported in the historical inventories, the number of events between 0.01 to 0.3 m³ that are deposited on the track or in the ditch (based on the model) is larger than reported in the historical inventories. This suggests that these inventories may contain an under sampling of small rockfall events (or that the frequency of smaller events on this individual slope is higher than the overall trend for the region). For rockfall volumes greater than 0.3 m³, the yearly frequency of events is lower than the historically recorded frequencies and could be a result of the short duration of TLS data collection (18 months).

These adjusted curves can be used to calculate the annual frequency of rockfalls deposited on the tracks that present a derailment hazard (rockfalls that land on the tracks and have a particle dimension between 0.3 and 1.0 m). Based on the curves shown in Fig. 10b, the frequency of events this size occurring on the slope is 997 rockfalls per year, but the frequency of events deposited on the track is only 56 rockfall events per year.

The rockfall deposition curves can be used to understand which rockfall source locations may produce rockfall events to exceed susceptibility criteria set out by rail operators. For example, based on the results presented in Fig. 11, if the railway were to accept a 10% probability of an event landing on the tracks, given the rockfall source location and volume, then areas of concern would be any rockfall sourced from 0 to 25 m distance. This example and other scenarios are outlined in Table 3.
Table 3. Locations and block sizes that exceed several conceptual probability thresholds

<table>
<thead>
<tr>
<th>Probability Threshold</th>
<th>Source Location/Volumes Exceeding Threshold</th>
</tr>
</thead>
</table>
| 5%                    | • All rockfalls sourced less than 50 m from track  
                         • Rockfalls greater than 0.1 m$^3$ sourced between 50 and 75 m from track  
                         • Rockfalls between 0.1 and 10 m$^3$ sourced between 75 and 100 m from track |
| 10%                   | • All rockfalls sourced less than 25 m from track |
| 20%                   | • Rockfalls greater than 0.1 m$^3$ sourced less than 25 m from track |

4.2 Railway Maintenance Planning

Similar to how these adjusted curves can be used to calculate the annual frequency of rockfalls deposited on the railway tracks, they can also be used to calculate the frequency of events deposited in the ditches. Relating the frequencies of events to volume, the expected total yearly accumulation of material in the ditches can be calculated (Fig. 10c). This will provide an estimate of the maintenance work that is likely to be expected in order to retain sufficient ditch collection capacity.

5 Discussion

We have presented a workflow for using 3D remote sensing data to identify rockfall locations, volumes and source zone characteristics and to perform slope classifications. These data can then be used to develop rockfall simulations which can help in understanding the likelihood of these events reaching track level. Using the classified 3D slope models, we can logically differentiate between different zones on the slope to estimate modelling parameters. The results of these models can be used to determine a more refined estimate of hazard frequency and to understand which areas of the slope may produce rockfalls that exceed susceptibility criteria set out by the rail operators. Recent studies have shown that it is possible to identify deformation of rock blocks and estimate their volume prior to failure, and this can be used to give warning to the railways of potential slope instabilities (Kromer et al., 2016). Understanding the likelihood of a block being deposited at track level based on its volume and source zone location can help to identify if a deforming block has the potential to present a hazard. It is important to note that these methods present the likelihood of a block being deposited on the tracks but do not consider blocks which may have sufficient energy to damage the tracks and cause derailment.

The MCF curve for rockfall source zones identified from TLS shows a power law fit for volumes greater than 0.02 m$^3$, however the curve based on rockfalls deposited in the ditch or at track level (based on probabilities from modelling) shows some rollover for volumes in the lower magnitude ranges. Many studies ( Hungr et al. (1999), Dussauge-Peisser et al. (2002) ) have reported that the rollover in historical rockfall inventories is due to a sampling bias in the lower magnitude ranges, however if the rockfall events occurring on the slope truly follow a power law, and smaller events are less likely to travel towards the bottom
of the slope, then an inventory based only on track and ditch inspections would be expected to show rollover in the lower volume ranges regardless of any sampling bias.

Data from the White Canyon has been used as a case study to develop and present the methods described in this paper, however these models have not been fully calibrated due to the large window between sampling periods, which may prevent us from identifying all individual rockfall events if multiple events occur in the same location during the sampling period (van Veen et al., 2017). It is also more difficult to map rockfall trajectories from the change detection results, to validate the models, when the duration between scans is increased. The overall duration of data collection has been limited to only 18 months, therefore the inventory is lacking in detail for larger-volume rockfalls which occur less frequently. As more data is acquired and processed, the modelling parameters can be refined, MCF curves updated, and hazard frequencies adjusted.

Similar to many standard rockfall modelling software packages, the RockyFor3D model does not take into account fragmentation of rock blocks upon impact or deposition. In addition, and as is the case with many large rock slopes which are difficult to physically access, we do not have sufficient data such as physical tests or high temporal monitoring to calibrate models of rockfall fragmentation on this slope. Therefore, the deposited volumes and block sizes are likely smaller than the source zone volumes. While it is possible to determine the energy statistics at a given cell in the RockyFor3D raster model, the ability to track individual rockfall events including their passages and energies at a given location is not possible. However, this capability can help to separate events that are deposited into talus channels versus rocks that are passing primarily over outcrop to better understand how rockfalls behave on this complex slope. Ongoing work involves the development of a fully 3D rockfall model using the Unity game engine (Ondercin, 2016) which will consider the influence of these factors (Sala et al., 2017). Additional work utilizing the inventory of rockfall events is focused on developing a logic for identifying rockfall source zones based on multiple factors, considering the slope geometry required to produce rockfalls of varying magnitudes. This will in turn be used to refine the rockfall simulations.

6 Conclusions

Remote sensing data can be used to collect data for hazardous rock slopes along rail corridors that may otherwise be inaccessible. Detailed geometrical data and high-resolution images derived from these methods can be used to create 3D slope classifications and assess general structural characteristics. By performing change detection, it is possible to extract rockfall information and create an inventory of the source zones, approximate volume and timing of the events. As shown in this paper, a rockfall source zone inventory can be used in its simplest form to assess the slope angle from which rockfalls are sourced, and to provide an assessment of the magnitude-frequency relationship for the slope. The challenge remains to understand which rockfalls sourced on the slope will make it to track level.
This study serves to outline a methodology and workflow that can be applied to rock slope stability and rockfall hazard assessment. Rockfall models can be used to determine the likelihood of a rockfall impacting the railways tracks given its volume and source zone location, which can be used to better understand the hazard likelihood and provide estimates of volumes that could accumulate in catchment ditches. The methods developed have the potential to incorporate new data as it becomes available and can be applied to additional slopes in the future.

The primary limitation of the methods described is incorporating rockfall fragmentation into the simulations, as it is difficult to calibrate fragmentation models for largely inaccessible slopes with discontinuous temporal datasets. In addition, many simulation programs do not effectively model fragmentation. As further improvements are made to simulation packages to accurately model rockfall fragmentation, the methods presented can be extended to additional software.

Contributions
Megan van Veen coordinated the project, analysed the data, and prepared the manuscript. Megan van Veen, D. Jean Hutchinson, and Matthew Ondercin developed the framework for the study. Matthew Ondercin and David Bonneau developed the simulation code with input from Zac Sala. David Bonneau ran the simulations. Matt Lato provided technical support throughout different stages of the study. All co-authors provided manuscript review. The authors declare that they have no conflict of interest.

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Ondercin, M.: An Exploration of Rockfall Modelling Through Game Engines, M.A.Sc, Geological Sciencees and Geological Engineering, Queen's University, 2016.


Figure 1. a) Location of the White Canyon b) Plan view of the White Canyon West and East slopes with the location of highway and rail lines indicated c) and d) Photographs of the White Canyon West illustrating complex geometry formed by deeply incised talus channels and vertical outcrops. Rock sheds are located at the base of several talus channels to guide material over the railway tracks.

Figure 2. a) Plan view of TLS model for the White Canyon West slopes b) Plan view of aligned TLS and ALS models for the White Canyon West slopes
Figure 3. Examples of the four main geological units in the White Canyon a) Quartzfeldspathic gneiss (Lytton Gneiss) b) red-stained granodiorite (Mt. Lytton Batholith) c) dioritic intrusions and d) tonalite dyke (outlined in red)
Figure 4. Classified slope models for White Canyon West based on: a) lithology b) ground cover type and c) GSI
Figure 5. a) Magnitude-cumulative frequency curves for rockfalls extracted from TLS compared to historical records (digitized from Hungr et al., 1999) b) Graph of rockfall frequencies based on source zone slope angles c) Spatial distribution of rockfall event locations from TLS data collected between November 11, 2014 and May 2, 2016.
Figure 6. Examples of slope areas for each of the classified GSI ranges. The $rg_{70}$, $rg_{20}$, and $rg_{10}$ input parameters to the RockyFor3D model for each GSI class are indicated. The 20-30 and 30-40 GSI examples are shown from the Mt. Lytton Batholith. All other images are the quartzofeldspathic gneiss unit.
Figure 7. a) Rockfall source zones used in RockyFor3D modelling, based on slope angle. b) Horizontal distances from track used for modelling to relate deposited blocks to their source zone.

Figure 8. Example of deposition outputs from the RockyFor3D model for the 250 to 300 m source zone. Coloured cells indicate rockfall deposition points and the colour of the cell represents the minimum volume deposited in each cell: for example, the darkest red colour represents cells in which only blocks $100 \text{ m}^3$ were deposited, and the second darkest colour represents cells in which blocks $10 \text{ m}^3$ or larger ($100\text{ m}^3$ were deposited).
Figure 9. Rockfall deposition percentages based on classified outputs from the RockyFor3D model showing a) percentage of rockfalls deposited in the ditch, on tracks, or past the tracks, b) in the ditch or on the tracks, c) on the tracks, d) in the ditch, and e) past the tracks (on the downslope side)
Figure 10. (a) Modified MCF curves for rockfalls deposited on track or in the ditch compared to TLS inventory and historical data, (b) modified MCF curves for rockfalls deposited on track compared to TLS inventory and (c) modified MCF curves for rockfalls deposited in the ditch compared to TLS inventory.
Figure 11. Rockfalls deposited on track with 5%, 10% and 20% exceedance criteria identified