

**~~Mass movements~~ Mass movement mapping inventory map  
and their impacts analysis on the upper Tayyah valley's  
bridge along Shear escarpment highway, Asir region (Saudi  
Arabia) using remote sensing data and field investigation**

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**Abstract.** Escarpment highways, roads and mountainous areas in Saudi Arabia are facing landslides that are frequently occurring from time to time causing considerable damage to these areas. Shear escarpment highway is located in the north of the Abha city (Tayyah valley). It is the most important escarpment highway in the area, where all the light and heavy trucks and vehicle used it as the only corridor that connects the coastal areas in the western part of the Saudi Arabia with the Asir and Najran Regions. More than 10,000 heavy trucks and vehicles use this highway every day. In the upper portion of Tayyah valley of Shear escarpment highway, there are several landslide and erosion potential zones that affect the bridge and the highway between Tunnel 7 and 8. In this study, different types of landslides and erosion problems were considered to access their impacts on the upper Tayyah valley's bridge along Shear escarpment highway

using remote sensing data and field investigation. These landslides and erosion problems have a negative impact on this section of the highway. Landslide and erosional features inventory map was constructed using visual interpretation of the high resolution satellite image and the 3-D image view. The results were aided by a detailed field investigation. This work is not well known in Saudi Arabia and may be considered as the pioneer work in landslide hazard analysis. In addition ~~of that~~ to the field study, the remote sensing based analysis indicated that the study ~~area have~~ areas have different landslides including planar, circular, rockfall, and debris flows. These landslides were evaluated in the current study and some mitigation strategies were considered.

*Keywords:* Landslides; Inventory map; Erosion; RS; GIS; Mitigation; Asir; Saudi Arabia

## **1 Introduction**

Landslides are one of the natural hazards that cause serious economic and live losses every year all over the world. They hit mountainous areas and highways from time to time due to different triggering factors such as intense rainfall, high groundwater pressures, seismic activity (earthquakes and blasting), rapid snow melting, volcanic activity, and various types of human activities (Franklin and Senior 1997; Guzzetti et al., 2008a, b; Baum and Godt, 2010; Iverson et al., 2011; Youssef et al., 2012). Cruden and Varnes, 1996; Shroder and Bishop, 1998; Regmi et al., 2013a, 2013b, indicated that landslides can be of many types such as: involve flowing, sliding, toppling, or falling, and complex landslides (a combination of two or more types of movements). In the current research the words “landslide”, “mass movement”, and “slope failure” are used as synonyms. In mountainous areas of the southern Saudi Arabia, there are lots

of urban areas, highways, and escarpment roads are prone to different types of landslides such as rockfalls, debris flows, and sliding (planar, wedge, and circular failures) (Youssef et al., 2012, 2014a). Among these landslide problems in Saudi Arabia; the most prominent ones are the Al-Hada debris flow in which occurred in August 2012 (Youssef et al., 2013) and Al-Raith debris flow in March 2013 (Youssef et al., 2014b). Many authors such as Petley (2008) and Van Westen et al. (2006) used different data sources such as field data collection, topographical and geological maps, and satellite images interpretation to prepare landslides inventory map. The work of Brabb (1991) can be considered as one of the pioneer work indicated that landslide in landslide inventory mapping represents the corner stone for several aspects such as documenting the occurrence of landslides phenomena in different areas (Trigila et al., 2010). Similarly, other studies related to landslide analysis are well documented in the literature such as; landslide hazard analysis (Harp et al., 2011a; Guzzetti et al., 2012), landslides susceptibility analysis (Cardinali et al., 2006, van Westen et al., 2006, 2008, Bălteanu et al., 2010, Pradhan and Lee, 2010, Pourghasemi et al., 2012; Xu et al., 2013a; Youssef 2015; Youssef et al., 2013, 2014a, 2014b, 2014c), hazard assessment (Pradhan and Lee, 2007, Xu et al., 2012a, 2012b), distribution of landslides in relation to morphological and geological characteristics (Guzzetti et al., 1996), as well as evaluating the areas that dominated by mass wasting processes (Guzzetti et al., 2008a, b, 2009, Parker et al., 2011). Guzzetti et al., (2012) indicated that a landslide inventory map portrays the location, numbers and the types of mass movements that have left discernable traces in an area. In the past, before the use of GIS techniques landslides were mapped as points (Plafker et al., 1971, Keefer, 2000). The landslide inventory point map can give some information about the special distribution but not much information about the landslide dimensions and volumes.

71 |       There are different methods ~~could be help in~~ can be employed for the preparation of  
72 | ~~preparing~~ landslide inventory maps among them geomorphological field mapping (Brunsden,  
73 | 1985), visual interpretation of stereoscopic aerial photos (Turnar and Schuster, 1996). However,  
74 | with the ~~advancementing~~ of computer stereoscopic vision the use of aerial photographs for  
75 | landslide mapping was expanding (Nichol et al., 2006). Detection of landslides from aerial  
76 | photographs ~~require~~ requires experience training persons and interpretation criteria and methods  
77 | (Antonini et al., 2002a). In arid and semi-arid areas the morphological appearance of landslides  
78 | is not concealed by vegetation (De Blasio, 2011). Guzzetti et al., (2015) indicated that landslides  
79 | can change the landuse / landcover areas and change and modify the visual properties of the land  
80 | surface. Accordingly, satellite sensors can detect and map these areas easily. Different types of  
81 | satellite images were used to detect and map landslides such as; ~~L~~andsat and SPOT images  
82 | (Gagnon, 1975; Huang and Chen, 1991; Vargas, 1992). Generally, tFerrain conditions are  
83 | mapped and used as an indication for the presence and/or absence of landslides (Lin et al., 2002).  
84 | Recently, the use of remote sensing technology for mapping and detecting of landslides has  
85 | increased dramatically. This is due to the availability of high and very high resolution satellite  
86 | sensors (passive and active types). In general, many authors used optical sensors images to map  
87 | and detect landslides using visual and analytical methods ~~among them~~ (Cheng et al., 2004; Lee  
88 | and Lee, 2006; Youssef et al., 2009; Martha et al., 2010; Fiorucci et al., 2011; Parker et al., 2011;  
89 | Youssef et al., 2013, 2014a, 2014b). Other authors used Synthetic Aperture Radar (SAR) to  
90 | detect, map, and monitor landslides ~~among them~~ (Ferretti et al., 2000; Hooper et al., 2004, 2007;  
91 | Guzzetti et al., 2009; Lauknes et al., 2010).

92 |       Several attempts ~~were used~~ have been made to map and detect landslides including the  
93 | interpretation and analysis of high and very high resolution satellite images, digital elevation

model (DEM), and 3D-image view. Pan sharpened images ~~were~~ have been applied for landslide mapping (Gao and Maroa, 2010; Fiorucci et al., 2011; Marcelino et al., 2009) whereas LiDAR DEM was used by Van Den Eeckhaut et al., 2007; Hanebery et al., 2009). In a recent paper, Harp et al. (2011a) indicated that DEM is an essential in landslide mapping. However, Xu (2014) indicated that DEM is less important because landslides can be detected based on ridges and drainages on remote sensing images. He also concluded that using DEM ~~for prepare~~ the 3D-image view of areas can be very powerful tool for detecting and mapping of landslides. In addition, very high resolution satellite images can be combined with high resolution DEM to provide a 3-D image view of the area and by visual interpretation landslides can be mapped and detected (Nichol et al., 2006; Bajracharya and Bajracharya, 2008; Youssef et al., 2009, 20013). High and very high satellite images represent a good and an accurate alternative data to the aerial photographs and can be used to detect and map landslides (Casagli et al., 2005; Marcellino et al., 2009; Youssef et al., 2013, 2014a, 2014b).– Recently, Gao and Maroa (2010); Fiorucci et al. (2011) compared the results of ~~using~~ high resolution satellite images and aerial photographs ~~for the~~ of similar resolution ~~and of the same~~ area to detect and map landslides. Their results showed that satellite images can provide similar and complementary landslide information. Xu (2014) indicated that analysis of the image texture and tone can differentiate and map landslide areas from the surroundings. However, this needs ~~the used images must be~~ continuous images and ~~cover covering~~ the entire area with, resolution ~~has to be~~ between high to very high, and free of clouds. Using the high-resolution satellite images, historical landslides could be observed as breaks in the highly vegetated area, bare soil, or geomorphological features, such as head and side scarps, flow tracks, and soil and debris deposits below a scar (De la Ville et al., 2002, Youssef et al., 2009).

Slope failures can be classified into two groups; first group is depending on the geometrical and mechanical nature of the discontinuities and the conditions of the rock masses which include Circular, Planar, Wedge, and Toppling failures (Farrokhnia et al., 2010; Regmi et al., 2014; Youssef et al., 2012; 2014b). The second group is rock failure by **rockfalls** and debris flows mechanism which cannot be analyzed using limiting equilibrium analysis. **Many authors studied the debris flows, their types, and mechanisms; (among them, Hungr, et al., 2001; Johnson, 1984; Pierson and Costa, 1987; Youssef et al., 2012, 2014b). Due to the high density and mobility of debris flows, they represent a serious hazard, which impose serious problems for people, properties, vehicles, and infrastructure in mountainous regions. Landslide types such as structural control, rockfalls, and debris flow need ~~a mitigation strategies~~mitigation strategies that may be required to minimize their risks which have been applied in many ~~research~~ areas (Frenez et al., 2004; Maerz et al., 2014; Rickenmann, 1999; Rimbock and Strobl, 2002; Youssef et al., 2012, 2014b).**

**In tThe current study, one of the main purposes of this paper is to emphasize the necessity to establish landslide and erosion features inventories, which would make the future landslide inventory maps more objective and consistent. This study deals with the evaluation, mapping, and determination of the characteristics of the different types of problems related to landslides and erosion features and their impacts on the bridge and highway section along the upper portion of Shear escarpment highway (Tayyah valley), which is located between tunnel 7 and tunnel 8. This area is a landslide prone area due to the adverse geological formation, structural features, steep slopes, drainage gullies and rills, highly dissected topography, and rainfall impacts. In this research, field investigation was done to cluster different slope failures that have a major impacts on the highway areas and bridge were studied in detail. In addition,**

different techniques and tools were used to prepare landslide inventory map for the study area using high-resolution satellite image and high resolution 3D-image view. The principles and techniques used include that all landslides should be mapped as long as they can be recognized from field surveys and images and both the boundary and source area position of landslides should be mapped. We consider methods and techniques for mapping the surface characteristics of landslides of different types, with some geometry and characteristics of slope failures. Most landslides in the study area were detected ~~according to the use of~~based on the field investigation data and interpretation of high resolution satellite image and 3D-image view. According to the use of the high resolution data the smallest scale of landslides was few meters length.

## 2 Study area

Shear escarpment highway is located in the Asir region, Saudi Arabia (Fig. 1a). It represents a part of Abha Highland (which is related to Arabian shield). It descends from the top of the escarpment (highly rigged mountains) near the Abha City down to the Mahail Asir then to the coastal zone of the western Saudi Arabia (Fig 1b). It connects the Red Sea coastal areas (western region of the Saudi Arabia) with the Asir and Najran regions. This escarpment road is one of the first roads in the area constructed through extremely rugged mountainous terrain about 32 years ago. It is an important escarpment highway, as it offers access to the private vehicles, light-duty trucks, and the only escarpment highway for the heavy duty trucks. The Shear escarpment highway is about 16 km long, measured from the top of the escarpment (2200 m above sea level (asl)) from east to the Mahail Asir city (approximately 700 m asl). The highway is characterized by the presence of about 11 tunnels and many vertical and horizontal curvatures as well as some bridges. The current study is carried out to deal with the bridge and the highway section (with a total length of 2150 m) located between tunnels 7 (at elevation of 1888 m) and 8

(at elevation of 2004 m) at the upper reach of Tayyah valley (Fig. 1b). The area is located on a small wadi that meets at a right angle with the main wadi of Tayyah (Fig. 1b). The small wadi that includes the study area is surrounded by high mountains with steep slopes. It appears as a deep and narrow gorge. This tributary (small wadi) flows with great force in steep and narrow channels often resulting in excessive toe erosion. The area is commonly prone to landslide activities (sliding, rockfalls, and debris flows) and erosion features due to running water through different gullies and rills. There are numbers of active landslides which are badly affecting the highway and bridge section and are the potential sites to cause disaster in the event of a major rainfall or earthquake.

**Fig. 1. About here**

### **3 Methodology**

In general practice, landslide hazard of an area is assessed by carrying out intensive field investigation, remote sensing data analysis, interpretation of geological and topographical data. This is usually accomplished by the analysis of several maps and landslide distribution of the area to classify them into various types. In the present work, assessment of landslide and erosion problems have been carried out with the help of different data types (Fig. 2). Lithological, morphological, hydrological, and structural characteristics of the study area might have influenced the distribution of landslides and erosional features. The geological and structural data were mapped according to the Abha quadrangle geological map (GM-75, 1: 250,000-scale). These geological and structural data were verified in the field. Many structural data were measured including joint planes and faults. All landslides, in the study area were identified and mapped using high resolution satellite image (QuickBirds 0.61 m spatial resolution from the year

2012), and 3D-image view and verified using intensive field investigation. The high resolution satellite image (QuickBird 0.61 m resolution) was overlaid on the high resolution DEM ~~was done to~~ prepare a 3D-image view (0.61 m resolution) ~~that will help in~~ in which was used to detecting and map landslides and erosional features in precise and accurate way. The high resolution DEM was ~~created~~ constructed from the interpolation of a digital topographic map 1:10,000 scale for the study area. These data were used in retrieving information related to topography, existing landslides, debris accumulation and other relevant features in relation to slope instability and erosion features. The satellite image was registered with reference to the topographic map of the study area by taking input ground control points from the image and reference points from the map. All the images were in UTM coordinate system, Zone 38, and WGS84 datum. Besides mapping the different types of landslides, rock mass rating (RMR) for different rock zones in the study area was identified to determine the quality of these rocks and to classify the study area into different zones. For that purpose, ~~Different~~ rock samples were collected from the ~~different~~ each landslide zones in order to apply rock shear test to determine the friction angle for rock plane sliding. Potential for planar failures was carried out using Dips 5 program (RocScience, 1999). Other types of failures such as circular, rockfalls, and debris flows were highlighted and mapped in the field. In addition, gullies, that dissect the study area, were mapped and different morphometric parameters were determined using watershed modeling system (WMS8.1). Different features of landslides and erosions were mapped using rigorous field investigation ~~and as well as~~ and as well as from the 3D-high resolution satellite image view for the study area. The remote sensing based analysis, field, and laboratory studies were coupled together to get the comprehensive view of the different types of landslide and erosion features that impose a high impact on the study area.

**Fig. 2. About here**

#### **4 Geomorphology, geological/structural setting and climatic characteristics**

Geomorphologically, the study area is located at the upper portion of Tayyah valley. The escarpment itself is the result of erosional retreat of uplifted Precambrian rocks that were elevated concurrent with initiation of rifting in the Red Sea during the late Paleogene era. The escarpment runs in different direction such as east – west and north – south. Whereas, the study area has a curvature shape (Fig. 1b).

Geologically; the study area is mainly located in the Bahah group within the Tayyah belt (Abha quadrangle GM-75; Greenwood, 1985) (Fig. 3a). The Bahah group is a major component in the western part of the Tayyah belt. It consists of a fault bounded blocks including abundant volcanic greywacke, local boulder conglomerate, carbonaceous shale, slate, chert, bedded tuff, and interbeds of volcanic flow rock. In the study area there are abundant of greywacke and slate. Greywacke is characterized by massive to thin bedded in form and has sedimentary structures including grading, cross bedding, and lamina bedding. Massive greywacke forms thick beds from 1 – 3 m and interlayered with fine grained and laminated bedded of slate which are strongly metamorphosed to green schist facies. The Bahah group rocks in the Tayyah belt are weakly to moderately cleavage where as they are highly cleaved near faults. They are characterized by the presence of one cleavage (schistosity) which has steep dips toward east or west. Some intrusive rocks including granodiorite and granite were encountered in the Tayyah belt. Near the intrusive contact amphibolite grade metamorphic rocks were encountered. Other rock units are encountered in the surrounding areas include, alluvium and gravel, basalt and andesite, biotite

monzogranite, biotite-quartzite-plagioclase granofels, hornblende-biotite tonalite and granodiorite, Jeddah and Bahah groups, and Muscovite –biotite tonalite and granodiorite.

The area is traversed by many faults where many shear zones are formed. These tectonic features are responsible for crushing and shearing of the rocks in the region. Different types of structures such as faults, folds and linear structures are encountered in the study area and its surroundings according to geological map (Abha quadrangle GM-75) (Greenwood, 1985) (Fig. 3a). The geological map was verified by field investigation. Along the main curvature of the study area there is a major fault that cut through the rocks (Fig. 3a). The materials along the fault zone are highly crushed and weathered whereas the rocks become highly sheared and jointed as the distance increased (Greenwood, 1985) (Fig. 3b, c).

Climatically, Saudi Arabia is classified as an Arid to Semi-Arid region according to the “World Map of Kopper-Geiger Climate Classification” (Peel et. al., 2007). The study area is characterized by mild summers and cold winters. According to the analysis of rainfall station (A130, operated by the Ministry of Water and Electricity (MOWE) which is located in the southwest of the study area by about 20 km. Rainfall is typically occurs in intense thunderstorms from March to May. The average monthly precipitations were 29.5, 46.5, and 64 mm for March, April, and May respectively. The average annual precipitation is reported as about 273 mm/year, with a maximum rainfall of 1043 mm occurring in 1997. The maximum precipitation happened in a day was 180 mm in 2004.

**Fig. 3. About here**

## **5 Results and Discussion**

### **5.1 Detailed field investigation**

Existing and potential landslide areas were identified through detailed field investigation along the upper portion of the escarpment highway and bridge section of Tayyah valley (between tunnel 7 and Tunnel 8) (Figs. 4 and 5). This includes determination of the RMR characteristics of the study area as well the different types of landslides. The rock characteristics along the study area were classified into three zones (Fig. 4) according to the application of the rock mass rating system. In the current study, the RMR system was used in the analysis of the rock masses along the study area. The system first designed to analyze the rock conditions in tunnels but it was modified later to analyze slopes and foundations. The RMR system was applied on the 10 stations along the study area (Fig. 6). Its value was computed, according to Bieniawski (1979), by adding rating values for five parameters including, (1) strength of intact rock, (2) RQD (measured or estimated), (3) spacing of discontinuities, (4) condition of discontinuities, and (5) water inflow through discontinuities (estimated in the worst possible conditions). The RMR value ranges between 0 and 100 has been calculated using VP EXPERT program developed by Ware Inc (1985-1988). Analysis results of rock mass rating RMR for all stations are shown in Table 1. The results indicate that there are three zones in the study area: 1) High foliated rocks are characterized by completely schistose and the RMR values range from 19 to 35 which is from poor to very poor rocks. The strength of these rocks are low to very low (Fig. 4, Table 1). 2) Fault zone is characterized by highly sheared rocks and mostly crashed, main debris flows are formed in this zone due to the presence of crashed materials and colluvial materials. The physical

characteristics of these materials are composed of some boulders up to 0.5 m in diameter embedded in gravelly and fine sandy materials. The RMR values range from 16 to 19 which is very poor rocks (Fig. 4, Table 1). 3) Moderately jointed rocks which are characterized by semi massive rocks, sometimes low to moderately strong and characterized by the presence of planar and **rockfalls** types of failure; they are intruded by some felsic dykes. The RMR values range from 65 to 74 which is good (Fig. 4, Table 1).

**Fig. 4. About here**

**Table 1. About here**

Landslides in the study area were mapped, identified, and classified into rock slides (planar and circular failures), rockfalls, and debris flows. The planar failure type is predominantly along discontinuities. The **rockfalls** and sliding failures (planar failure) are mainly located in zone 1 and &3 (Fig. 5a, b, c). In the current study three sites ~~impose~~ have encountered **planar** failures from time to time and they were thoroughly examined. These failures are located in Bahah group which is characterized by weakly to moderately cleavage and highly cleaved near fault zone. Sometimes they are characterized by the presence of main joint set (schistosity) which has steep slope toward the bridge section as in site 3. Some intrusive and volcanic dykes were encountered in the Tayyah belt. These gives large **planar** failures along the main joint set that dips toward the bridge section as shown in sites 1 and 2. Data collected from these three

298 sites, including the main joint set for each site that dip toward the bridge section, were plotted on  
299 stereonet (Fig. 6). Sites 1 and 2 are located in moderately jointed zone and their main joint sets  
300 are characterized by a dip directions ranges from  $7^{\circ}$  to  $17^{\circ}$  and dip angle from  $48^{\circ}$  to  $59^{\circ}$  (Table  
301 | 2, Fig. 7). Field investigations, for these two sites indicate that both are ~~examined~~ large planar  
302 failures. By comparing the strike of the bridge section and these two locations indicated that they  
303 | are nearly parallel to each other. In addition, the site 3 is located in highly foliated rocks showing  
304 a shestosity texture and the main joint set has a dip direction of  $5^{\circ}$  and dip angle of  $80^{\circ}$  which is  
305 | parallel to strike of the bridge section. The dip/dip direction measurements ~~that~~ which was  
306 collected from these three sites were plotted on stereonet using Dips 5 software (RocScience,  
307 | 1999). Stereographic analysis alloweds ~~investigators-us~~ to visualize and measure discontinuities  
308 | in three-dimensions by projectingon ~~discontinuity~~ planes through a sphere and observing the  
309 trace of the line of intersection of the plane and sphere (Fig. 7). A structural control stability  
310 analysis utilizing the Markland Test Plot method, was used to assess the potential for planar  
311 sliding along the identified discontinuities. Markland test plots show the discontinuities in  
312 relation to potential planar sliding surfaces on a lower hemisphere stereonet projection. The slope  
313 face is shown as a marked great circle and the measured friction angle is represented by an  
314 interior circle. Based on discontinuity roughness and other properties of the rock, friction angles  
315 of the collected samples from the three sites have been measured using different techniques  
316 including 1) Rock data analysis of the field rock mass characteristics; and, 2) Rock shear box for  
317 the samples along the critical joints in these sites. The lowest friction angle and dip direction of  
318 the joints and rock cut were used to determine the potential planar failure. If discontinuity dip  
319 vectors plot within the shaded areas of the test plot, failure along the discontinuity is  
320 kinematically possible. Table 2 shows the different characteristics of each site and Fig. 10 shows

the stereonet presentations of the main discontinuity data collected from the rock cut stations above the bridge section of the study area. In the current study lowest measured friction angle of 35°, 40°, and 30° was used for these three sites respectively (based on the shear strength and rock data analysis) Table 2. The dip vectors of these three main joints sets occur within the crescent shaped shaded area, in addition the strike of these main joints have an angle less than 20° from the strike of the rock cut face and so planar failure for these main joints are potential.

**Fig. 5. About here**

**Fig. 6. About here**

**Table 2. About here**

**Fig. 7. About here**

In zone 2 (fault zone), field investigations showed that the area is highly affected by fault and most of the rock in the area is highly jointed, weathered, and crashed. These highly crashed rocks are mixed together and with the colluvium soils that located with different sizes (Fig. 5d, k). Generally, circular failures have no structural pattern and the failure surface is free to find the line of least resistance through the slope and the failure geometry is circular (Hoek and Bray, 1981; and (Hoek, 1982). Many circular failures were detected in the study area (zone 2) and some of them are clearly appeared in Fig. 5d, e. Whereas other circular failures are new where some tension cracks begin to be appeared at the upper portion of the bench located above the bridge level (Fig. 5f, g). Sometimes complex circular failures are detected in which multiple

failure modes, many tension cracks, and subsidence are located along the highly sheared and colluvial materials above the bridge level.

Rockfalls are occurred due to a combination of different factors and not related to the structures (joint planes). In most of the rock cuts and slopes, rockfalls are difficult to analyze. Badger and Lowell (1992) mentioned that large number of accidents and about half dozen fatalities were related to rockfalls in the last 30 years. In the study area, most of rockfalls are related to the effect of undercutting of the weak materials or due to sliding effect and leave other blocks hanging over, others related to erosional effect of rainfall especially in debris and colluvium materials where the weak materials eroded and leaving large blocks without any support (Fig. 5h, i). With the effect of gravity, rainfall, and vibration due to heavy trucks, these overhanging materials will fall down.

In the study area, the debris flows are mostly confined along natural drainage lines as well as along the fault zone. Debris flows are occurring along the gullies with an average slopes that vary from  $13.2^\circ$  for channel (1) to  $32.2^\circ$  for channel (7) (Figs. 5a, j, k & 9). Most of the debris flows occur along the gullies where loose overburden materials on such slopes, when saturated during rains causes debris flows. This happened very often and these debris flows have an erosion effect along the gullies and between the bridge piles. Where most of weak materials, highly jointed rocks, and colluvial soils erode and moved downwards with running water. The debris flows from these gullies extend below the road and bridge level to the main wadi. Figure 5a, j, k show some examples of debris flow channels and erosion features along the gullies in the study area.

Other type of threatening problem that is related to the erosional effect of the running water through the drainage channels (gullies) that cut through the mountain and run under the

bridge and through culverts. There are many drainage channels (gullies) ~~that were~~ found in the study area that impose erosion impact under and between the bridge piles and under the culverts (Fig. 5a, j, l, m). The erosional and debris flows could be a problem in the future and will pose threat to the stability of the bridge and cause damages to vehicles and disrupts traffic.

## 5.2 Erosion problems under and between the bridge piles

Many authors focused their studies on rill and inter-rill erosion (Poesen and Hooke, 1997). Others focused on gullies erosions and they indicated that these gullies represent the main sediment source in Mediterranean environments (Casali et al., 1999; Poesen et al., 2002, 2003; Valcarcel et al., 2003). The erosion processes in the study area have a severe effect in the areas between bridge piles and the area along the drainage channels (gullies). In the current study, detailed drainage network were drawn from the high resolution satellite image and filed investigations and were compared with the networks that extracted from SRTM 90 m and Digital elevation model of 5 m resolution (created from topographic map of 1:10,000) using watershed modeling system (WMS 8.1) (Fig. 8). Different types of morphometric parameters were determined for each gully to determine its activity ~~in-on~~ erosion effect (Table 3). Existing and potential erosion areas were identified through field investigation along the study area and by using high resolution image. The erosion materials can cause the debris flow to occur after the gradual increase in discharge. Width of the existing gullies ranges from 6 to 15m whereas the depth of erosions was determined to be from 2 to 5 m (Table 3). Field investigation indicated that most of the gullies ~~haveare~~ cut through foliated rocks (zone 1) which includes channels 6, 7, 8, 9, and 10, and the fault zone (zone 2) which includes channels 2, 3, 4, and 5. However, few gullies are located in moderately jointed rocks (zone 3) which include channel 1. In the study area most

of rocks here are highly foliated (metamorphic), sometimes intruded by different dykes (of acidic igneous rocks). These rocks are overlaid by loose residual soils and slope wash. After the rainfall and with continuities of debris flow, the loose soil cover (debris materials and crashed rocks along the fault zone) are moved away and bare rocks are now exposed on the side walls and at the bottom of the gullies (Fig.5a, j, k, l, m). At the surface of the rocks, and between the bridges piles there are scouring effect (erosions). These debris coming from these areas moved with water toward the main wadi course. Data analysis and field investigation indicated that there are three factors that play a major influence in the erosion processes and which are claimed to be the most important causes of channel erosion. These factors include high runoff due to intense rainfall, weak materials that is located along the gullies, and the steepening slope of these gullies (Table 3).

**Fig. 8. About here**

**Table 3. About here**

### **5.3 Landslide and erosion inventory map**

In the current work, the computer screen based visual interpretation of high resolution satellite image (QuickBird image 0.6m resolution) and the 3D-image view was used in landslide identification and mapping on a GIS platform (Fig 4, 9a, c, d). Landslides attributed database was prepared for the study area (vector landslide inventory map). About 000 landslides and

different erosional features were mapped. The data was verified according to the extensive field investigation. Different types of landslides were detected in satellite imagery according to special characteristics of the landslides and the erosional features. These features include erosion features, scarps, slides, materials size, shape, tone contrast and morphological expression, and fallen materials (Fig. 9). Many potential landslide zones (rockfalls, rock sliding, circular failures, and debris channels) and erosion problems were mapped and investigated ~~on~~ using the high resolution satellite image using ArcGIS 10.2 (Fig. 5, 9). The high resolution satellite image and the high resolution 3D-image were compared with the field investigation for the same area (Fig. 9a, b). The areas affected by landslide showed high differences in their tone than the surrounding materials as well as in some instances there are fallen materials under the landslide areas (Fig. 9). Areas with landslides have typically elliptical in shape. Field checking was carried out and corrections were incorporated on the image to draw the boundary lines of the landslides. These different types of landslides and erosion features along the study area are shown in Figs. 5, 9. The active portions of the landslides as observed in the field were considered as problematic areas which located above the highway and the bridge piles. All data were collected and assembled together ~~using Arc-GIS 10.2~~ to create a landslide and erosion inventory map of the study area (Fig. 10). This final map shows the distribution of different types of landslides and erosion features problems in the study area including locations of debris flows, rockfalls, translational sliding, few rotational failures and erosional features along different gullies. Table (4) shows the statistical distribution of these different types of landslides according to the type of investigation and analysis. Data shows that according to the field investigation 77 landslide locations were detected and from the satellite image interpretation and 66 landslides were

mapped. Some of these landslides have a direct impact on the highway and bridge section in the study area.

**Fig. 9. About here**

**Fig. 10. About here**

**Table 4. About here**

## **6 Mitigation strategies**

From engineering point of view there are various types of measures that can be used to reduce the impact of landslides on the highway (Bridge section) section between tunnel 7 and tunnel 8. An outline of different mitigation methods for debris flows and landslides as potential methods were given by different authors (deWolfe, 2006; deWolfe et al., 2008; Franzi et al., 2011; Huebl and Fiebiger, 2005; Maerz et al., 2014; Wagenbrenner et al., 2006; You et al., 2012; Youssef et al., 2012, 2014b).

In the current study different mitigation and remediation techniques could be used from future landslides and erosional flows. The generation of debris flow, runoff erosion, sliding failures, and rockfalls processes acting on the entire study area were taken into account. For the current study, the slides, rockfalls, debris flows and running water cause different type of problems. The effectiveness of landslide and erosion control treatments in the study area has been evaluated. Thus the aim of mitigation is to prevent different types of landslides and running water effect (erosion problems between bridge piles), which includes reducing the velocity of water flow, preventing down cutting erosion and decreasing the gradient of the gully. The mitigation methods proposed in the current study include:

1) Controlling the landslides by applying a suitable remediation/mitigation technique. Slope stabilization has to be done for the rock cuts and slopes above the bridge and highway level, this will reduce the volume of the initial material. For the unstable faces, shotcrete (the sprayed concrete process) have to be applied. Drainage ditches has to be established above the potential failures to divert the water and prevent infiltration into potential unstable areas, and benches have to be cleaned and established below the potential failures to increase the space to accommodate the falling rocks.

2) ~~for~~For the gullies with the effect of debris flows and erosion features, land management techniques have to be applied to decrease the erosional features by runoff diversion from the gullies at different levels along the benches. In the areas surrounding the bridge piles and under the culverts from up and down streams sides a layer of shotcrete need to be established in order to protect the area from scouring effect and protect the piles and culvert from any damage. In addition along the gullies grid dams need to be installed to reduce the velocity of water flow by decreasing the gradient of the gully and to stabilize slopes. This will provide barriers against runoff to reduce the erosion and resulting in a reduction for erosion potential.

## **7 Conclusions**

In the upper portion of Tayyah valley in Asir region, Saudi Arabia, there are many active landslides and erosion features (due to runoff and debris flows along the gullies that dissect the study area) particularly along the escarpment road (highway and bridge section) between tunnels 7 and 8, which are not only threatening human lives, but also causing damages to highway and bridge foundation. Rainfall in the study area can cause different types of landslides such as debris flows along the existing gullies that will increase the erosion effect along these gullies. These debris flows and erosion effect will impact the areas under and between the bridge piles

and under the culverts making undercutting features. In this study, a detailed study along the upper portion of Al-Tayyah escarpment highway between tunnel 7 and tunnel 8 was conducted to prepare landslide and erosion features inventory map. Field investigation indicated that the study area is classified into three zones, according to the RMR and the geological engineering characteristics of these zones. Zone (1) is characterized by high foliated rocks (Schistose rocks) and it is dominated by planar and rockfalls type of failures; zone (2) (fault zone area) is characterized by sheared and crashed rocks and this zone is dominated by circular type of failure; and zone (3) is characterized by moderately jointed rocks and this zone is dominated by planar and rockfalls types of failures. Debris flows and erosion features along the gullies are distributed in all zones and it is more effective and high dense in zone (2) due to its lithological and structural characteristics where most of this zone is sheared and crushed materials due to the fault. As well, different types of landslides were detected including planar sliding, circular failures, rockfalls and debris flows. Landslide and erosion features inventory map was prepared according to a coupling of the interpretation of high resolution satellite image, 3D-image view and field surveys. The results show that at least 143 landslides and erosional features were identified and mapped in the study area. These landslide and erosional features inventories have great significance for the ~~research~~ detail assessment of landslide susceptibility, distribution, and hazard assessment, as well as for the prevention and mitigation techniques. Different types of mitigation techniques have been proposed to protect, minimize, and/or prevent the impact of these landslides and runoff erosional features of the gullies on the study area.

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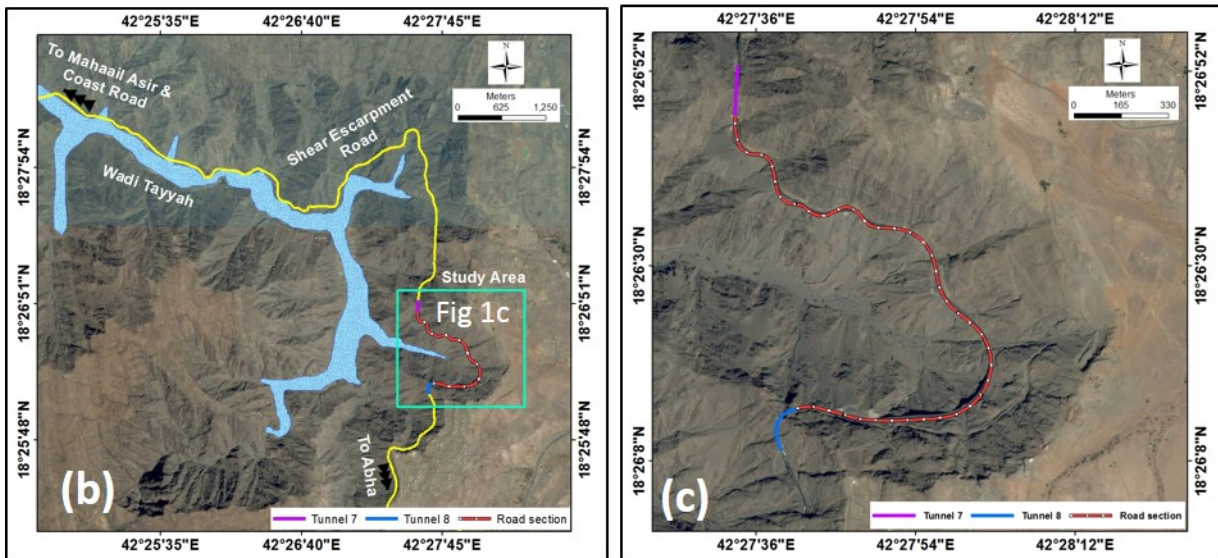
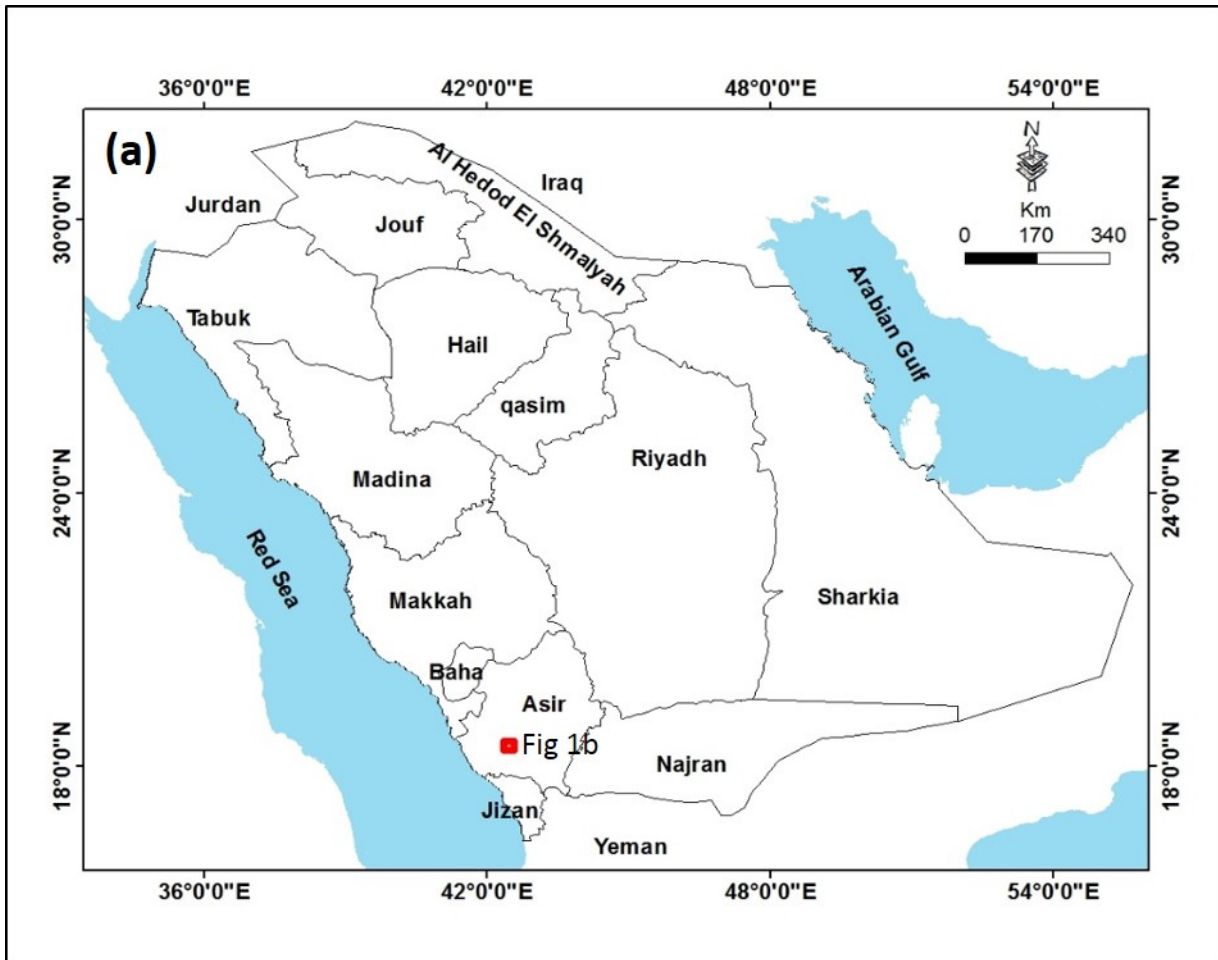
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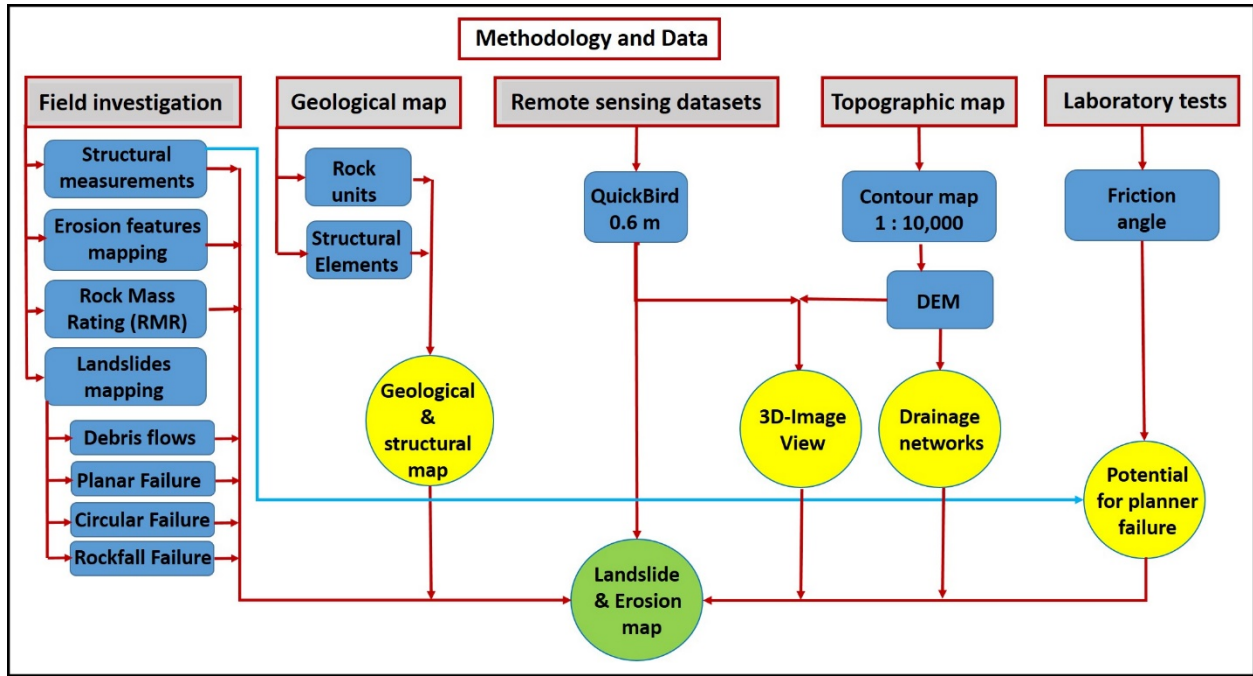
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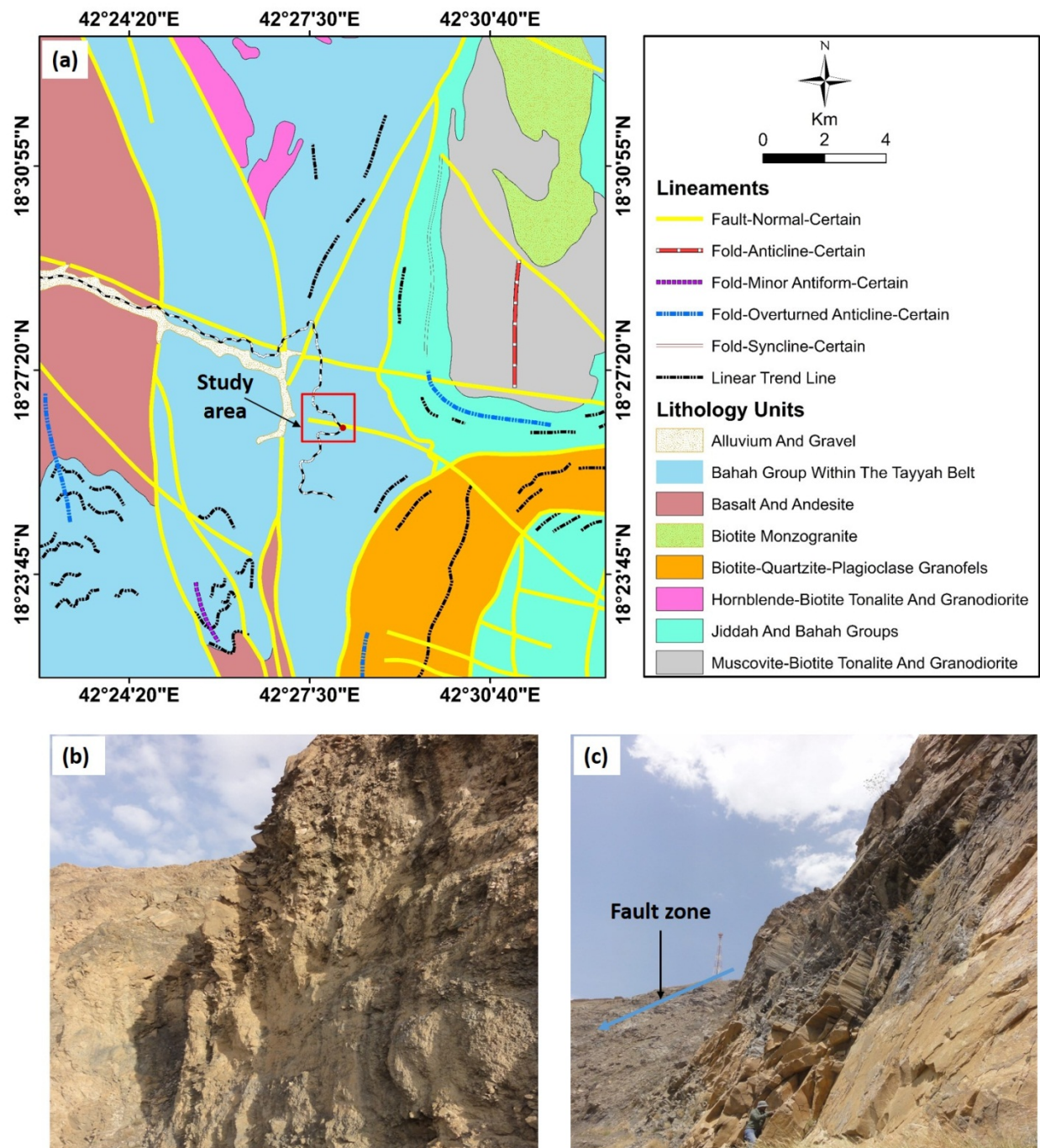
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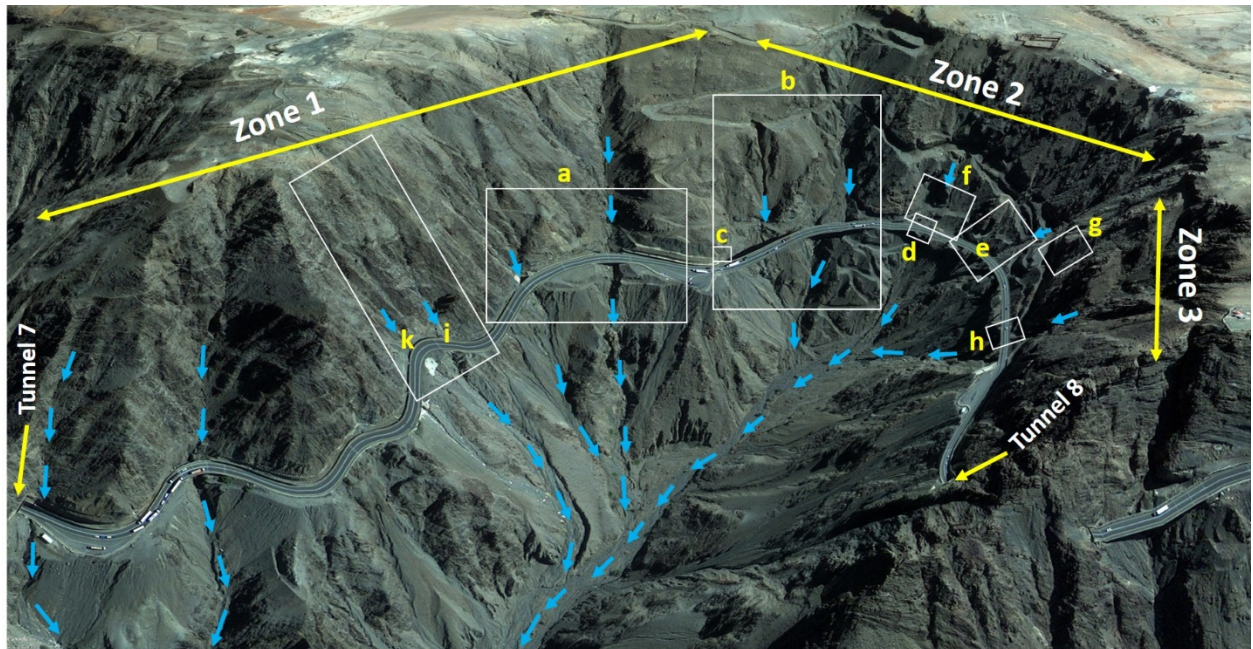
**Fig. 1.** a) Location of the study area in the KSA map. B) Upper portion of Tayyah Valley including the study area. C) Study area in a close up view.



**Fig. 2.** Flow chart showing data used and methodology in the study area.



**Fig. 3.** a) Geological map of the study area and its surroundings at the upper portion of the Tayyah Valley, b) rocks are highly crashed along the fault zone, and c) rocks close to fault zone are highly sheared and jointed.

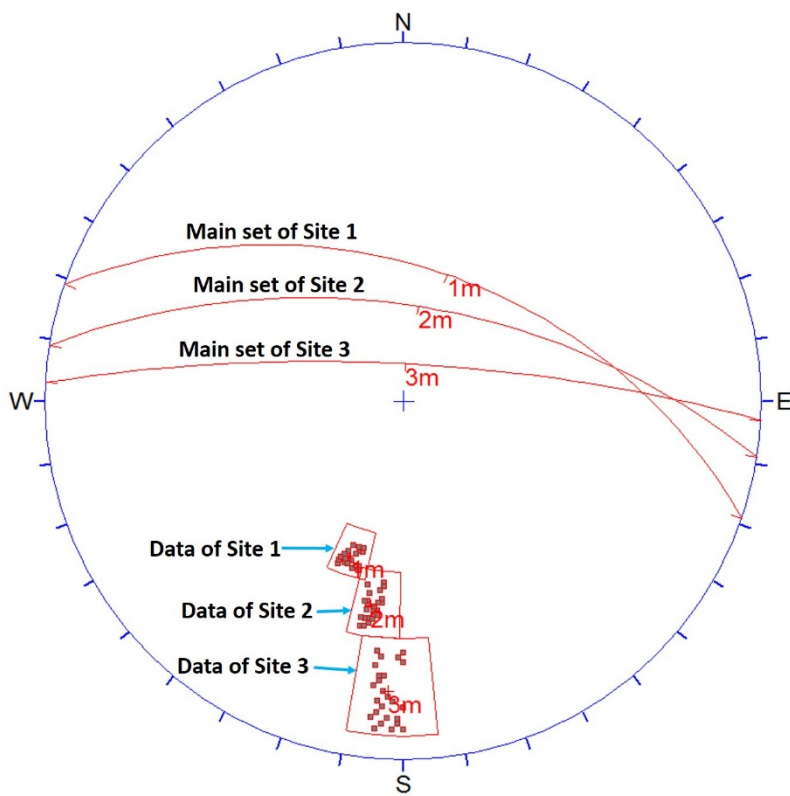


**Fig. 4.** 3D image view showing the potential gullies causing debris movements and erosions under and between the bridge piles as well as the areas for different types of landslides. 3 D image was prepared using QuickBird imagery. Note, the study area between tunnels 7 and 8 shows three zones and different landslides locations can be easily recognized, letters a to k are pictures in figure 5.

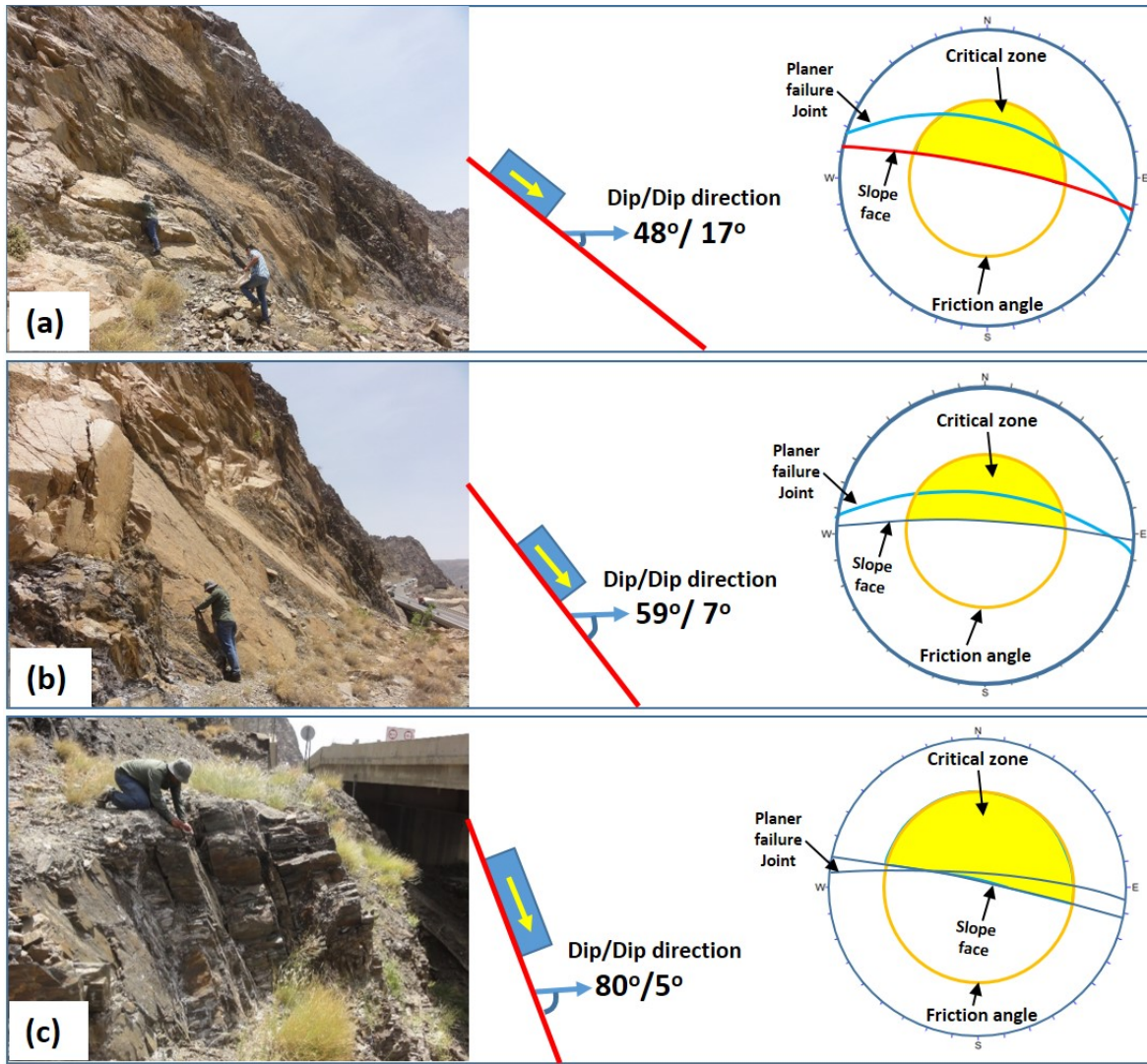


**Fig. 5.** Field photographs showing different types of landslides and erosional features along the highway and bridge section of the study area. a) sliding and erosional features due to running water along gullies, b) Sliding blocks along sliding plane (there are big blocks close to the bridge pile), c) large planar sliding, d) Potentially circular failure area where some circular failure

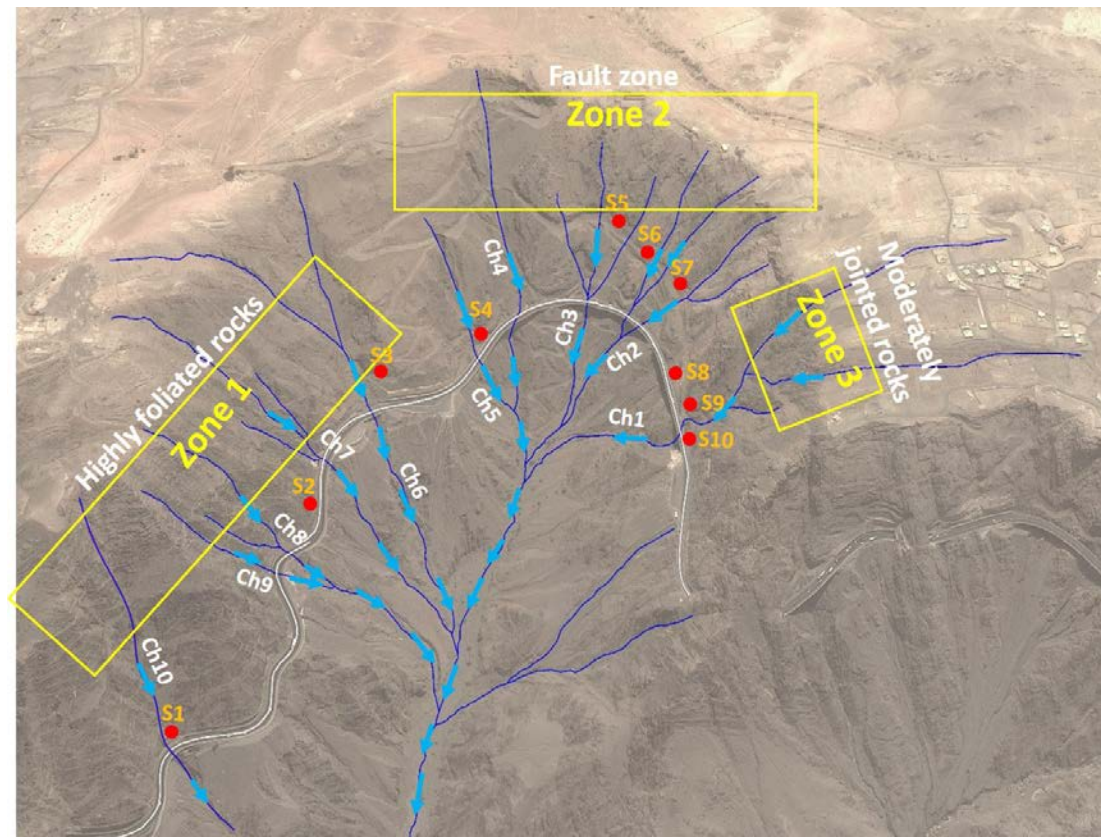
happened and many curved tension cracks appeared, e) circular failure very close to the bridge, f&g) Examples of curved tension cracked. h&i) Potentially rockfall failure area where some overhanging blocks appeared, j) debris channels along them erosional features and big boulders appear, k) debris materials of different sizes range from sand size up to big boulders 0.5 m in diameter, l) deep erosion and remove the materials surround one bridge pile, m) running water remove materials under the culvert make them under risk.



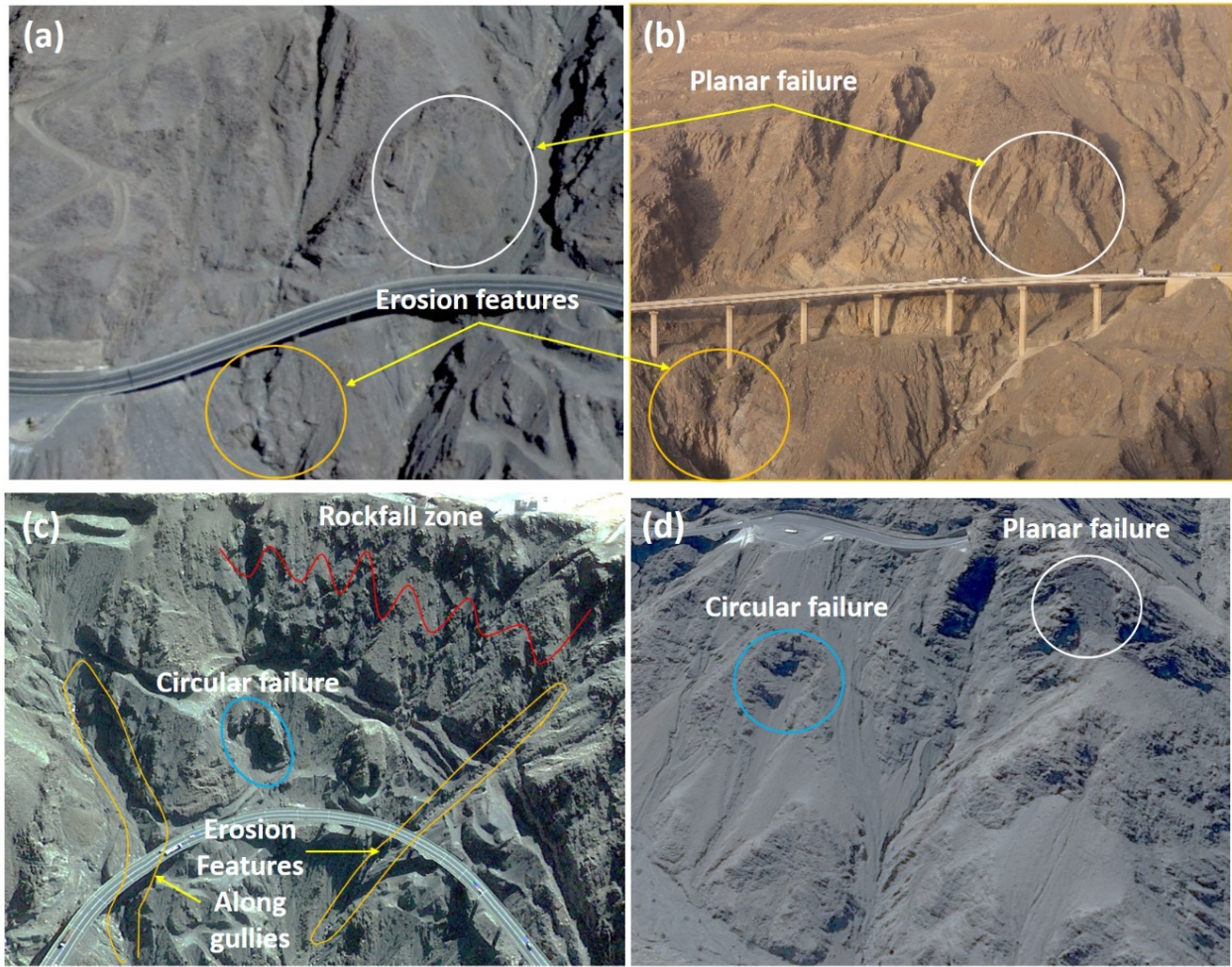
**Fig. 6.** Pole Plots for the data collected from the three sites.



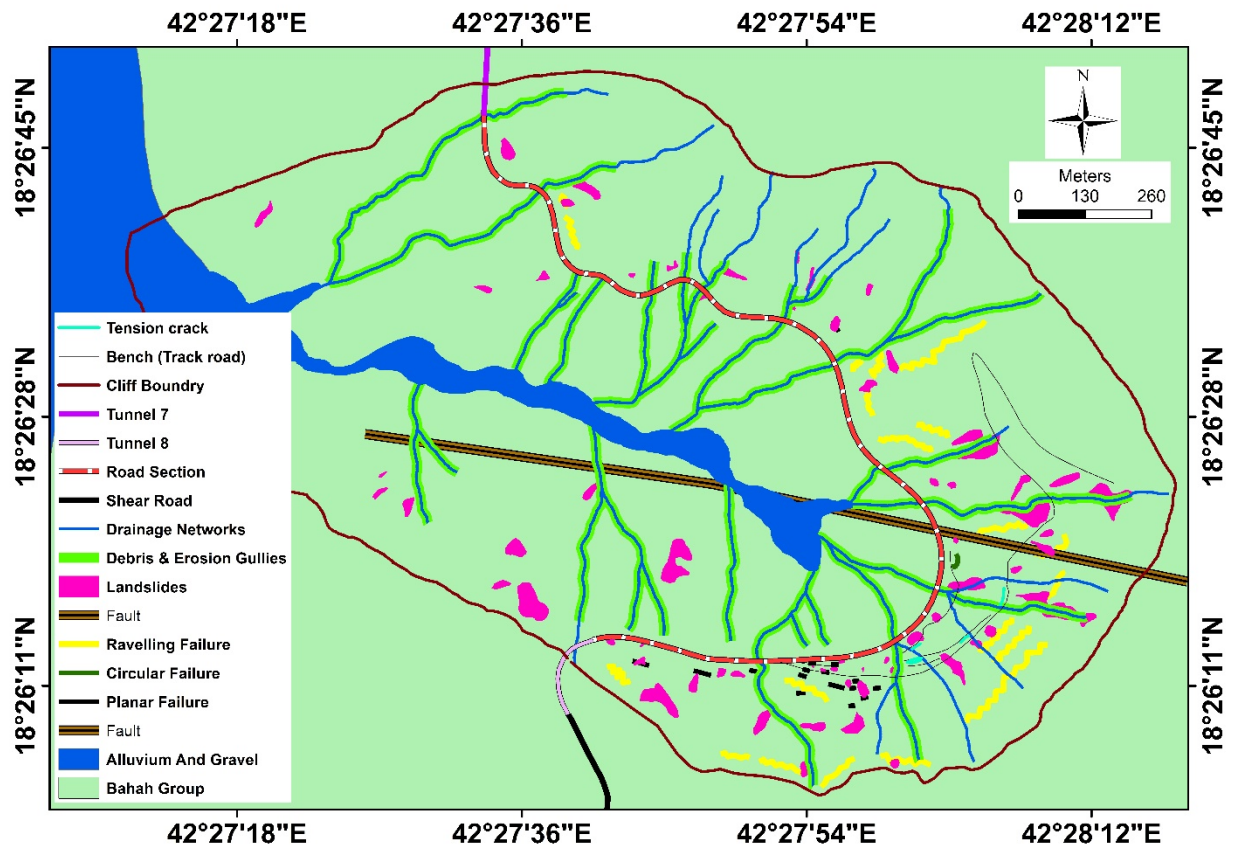
**Fig. 7.** The analysis used in the study for planar failures along the road section of the study area: (a, b, c) Field photographs at the three locations 1, 2, 3 respectively showing the planar joints dips toward the road section, simple sketch showing the dip/dip direction average values of plane that responsible of plannar failure for each site, and Markland Test circles showing the main set, friction angle, and rock cut face for each location plotted in Dips 5 program (note that there is potential planar failures as the plot vector of planes are located in the critical zone).



**Fig. 8.** Different gullies were mapped in the study area as well as the stations for RMR calculation is shown as red color dots.



**Fig. 9.** a) Different landslides features can be detected using high resolution satellite image; b) same landslides features appeared in field photograph in the same area; c&d) planar, circular, rockfall zone and erosion features as they appear in high resolution satellite image (3D images)



**Fig. 10.** Inventory map of the study area showing special distribution of different types of landslides and the erosional features (potential areas for rockfalls, planar failures, debris channels, circular failures, tension cracks and erosional features along the gullies that dissect the study area) due to visual inspection and field work.

**Table 1. RMR values for different stations along the study area**

<u>Zone #</u>	<u>Station #</u>	<u>UCS MPa</u>	<u>RQD</u>	<u>Spacing of discontinuities</u>	<u>Condition of discontinuities</u>	<u>Water condition</u>	<u>RMR basic</u>	<u>Rock Class</u>
<u>Z1</u>	<u>1</u>	<u>1-5</u>	<u>&lt;25</u>	<u>&lt;60 mm</u>	<u>Soft gouge &gt;5 mm thick Or Separation &gt; 5 mm Continuous</u>	<u>Damp</u>	<u>19</u>	<u>Very poor rocks</u>
	<u>2</u>	<u>3-25</u>	<u>25- 50</u>	<u>60-200 m</u>	<u>Slickensided surfaces Or Gouge &lt; 5 mm thick Or Separation 1-5mm</u>	<u>Wet</u>	<u>35</u>	<u>Poor rocks</u>
	<u>3</u>	<u>3-25</u>	<u>&lt;25</u>	<u>&lt;60 mm</u>	<u>Slickensided surfaces Or Gouge &lt; 5 mm thick Or Separation 1-5 mm</u>	<u>Damp</u>	<u>30</u>	<u>Poor rocks</u>
<u>Z 2</u>	<u>4</u>	<u>1-5</u>	<u>&lt;25</u>	<u>&lt;60 mm</u>	<u>Soft gouge &gt;5 mm thick Or Separation &gt; 5 mm Continuous</u>	<u>Damp</u>	<u>19</u>	<u>Very poor rocks</u>
	<u>5</u>	<u>1-5</u>	<u>&lt;25</u>	<u>&lt;60 mm</u>	<u>Soft gouge &gt;5 mm thick Or Separation &gt; 5 mm Continuous</u>	<u>Wet</u>	<u>16</u>	<u>Very poor rocks</u>
	<u>6</u>	<u>1-5</u>	<u>&lt;25</u>	<u>&lt;60 mm</u>	<u>Soft gouge &gt;5 mm thick Or Separation &gt; 5 mm Continuous</u>	<u>Wet</u>	<u>16</u>	<u>Very poor rocks</u>
	<u>7</u>	<u>1-5</u>	<u>&lt;25</u>	<u>&lt;60 mm</u>	<u>Soft gouge &gt;5 mm thick Or Separation &gt; 5 mm Continuous</u>	<u>Damp</u>	<u>19</u>	<u>Very poor rocks</u>
<u>Z3</u>	<u>8</u>	<u>50-100</u>	<u>50-75</u>	<u>0.6-2 m</u>	<u>Slightly rough surfaces Separation &lt;1 mm. Highly weathered walls</u>	<u>Damp</u>	<u>65</u>	<u>Good</u>
	<u>9</u>	<u>100-250</u>	<u>50-75</u>	<u>0.6-2 m</u>	<u>Slightly rough surfaces Separation &lt;1 mm. Highly weathered walls</u>	<u>Damp</u>	<u>70</u>	<u>Good</u>
	<u>10</u>	<u>100-250</u>	<u>75-90</u>	<u>&lt;60 mm</u>	<u>Slightly rough surfaces Separation &lt;1 mm. Highly weathered walls</u>	<u>Damp</u>	<u>74</u>	<u>Good</u>

**Table 2. Shows different characteristics of each site**

<u>Site Number</u>	<u>Main Joints</u>		<u>Rock Cut face</u>		<u>Friction angle (Ø) due shear test</u>
	<u>Dip angle</u>	<u>Dip direction</u>	<u>Dip angle</u>	<u>Dip direction</u>	
<u>1</u>	<u>48°</u>	<u>17°</u>	<u>80°</u>	<u>12°</u>	<u>35</u>
<u>2</u>	<u>59°</u>	<u>7°</u>	<u>80°</u>	<u>2°</u>	<u>40</u>
<u>3</u>	<u>80°</u>	<u>5°</u>	<u>85°</u>	<u>13°</u>	<u>30</u>

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**Table 3.** General characteristics of the gullies and different rock zones

	<u>Ch1</u>	<u>Ch2</u>	<u>Ch3</u>	<u>Ch4</u>	<u>Ch5</u>	<u>Ch6</u>	<u>Ch7</u>	<u>Ch8</u>	<u>Ch9</u>	<u>Ch10</u>
<u>Maximum elevation (m)</u>	<u>2163</u>	<u>2153</u>	<u>2145</u>	<u>2139</u>	<u>2136</u>	<u>2035</u>	<u>2127</u>	<u>2149</u>	<u>2113</u>	<u>2111</u>
<u>Minimum elevation (m)</u>	<u>1920</u>	<u>1964</u>	<u>1925</u>	<u>1835</u>	<u>1844</u>	<u>1843</u>	<u>1707</u>	<u>1793</u>	<u>1842</u>	<u>1877</u>
<u>Elevation difference (M)</u>	<u>243</u>	<u>189</u>	<u>220</u>	<u>304</u>	<u>292</u>	<u>192</u>	<u>420</u>	<u>356</u>	<u>271</u>	<u>234</u>
<u>Length (m)</u>	<u>826</u>	<u>526</u>	<u>319</u>	<u>783</u>	<u>588</u>	<u>360</u>	<u>587</u>	<u>831</u>	<u>598</u>	<u>538</u>
<u>Tan (θ°)</u>	<u>0.294</u>	<u>0.359</u>	<u>0.690</u>	<u>0.388</u>	<u>0.497</u>	<u>0.533</u>	<u>0.716</u>	<u>0.428</u>	<u>0.453</u>	<u>0.435</u>
<u>Slope Degree (θ°)</u>	<u>13.2°</u>	<u>16.2°</u>	<u>31.0°</u>	<u>17.5°</u>	<u>22.4°</u>	<u>24.0°</u>	<u>32.2°</u>	<u>19.3°</u>	<u>20.4°</u>	<u>19.6°</u>
<u>Width (m)</u>	<u>15</u>	<u>11</u>	<u>9</u>	<u>8</u>	<u>7</u>	<u>10</u>	<u>8</u>	<u>8</u>	<u>6</u>	<u>8</u>
<u>Depth of erosion (m)</u>	<u>Up to 2 meters</u>	<u>Up to 5 meters</u>				<u>Up to 3 meters</u>				
<u>Zone Name</u>	<u>Moderate jointed rocks</u>	<u>Fault zone</u>				<u>High foliated rocks</u>				
<u>Main characteristics</u>	<u>Planar failures and rockfalls</u>	<u>Circular failures and debris flows</u>				<u>Planar failure</u>				

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**Table 4.** Landslide types and numbers according to the used method

<u>Study Type</u>	<u>Landslide Type</u>	<u>Number</u>
<u>Satellite image and 3D-image view Interpretation</u>	<u>Landslides (planar, circular, rockfalls, debris flows)</u>	<u>66</u>
	<u>Planar</u>	<u>17</u>
	<u>Circular</u>	<u>11</u>
	<u>Rockfalls</u>	<u>20</u>
	<u>Debris and erosion features along the gullies</u>	<u>29</u>
<u>Total Number of landslides and erosion features</u>		<u>143</u>

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